

# **EQUIPMENT SEISMIC ANCHORAGE – THERMOFISHER SCIENTIFIC FREEZERS**

**2015-932-DC-001  
REV. 0**

## **PREPARED FOR:**

### **THERMOFISHER SCIENTIFIC**

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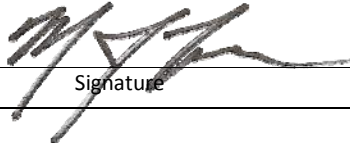

### **TOBOLSKI | WATKINS ENGINEERING, INC.**

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TWEI Contract: 2015-0932-CO-001, rev. 0  
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# AUTHORIZATION FOR RELEASE OF CALCULATION

Calculation Title: <b>Equipment Seismic Anchorage – ThermoFisher Scientific Freezers</b>		
Calculation Number.: 2015-0932-DC-001	Rev. No.:0	
Analyzed System: ThermoFisher Scientific Freezers		
Total Number of Pages (including this cover sheet): 284	Total Number of Attachments: 1	
Purpose of Revision: <b>Initial Issue</b>		
<b>ORIGINATOR</b>		
James Linjun Yan, PhD, PE Print Name	 Signature	07/13/2015 Date
<b>REVIEWER</b>		
Matthew Tobolski, PhD, SE Print Name	 Signature	07/13/2015 Date
<b>INDEPENDENT REVIEWER (if required)</b>		
NA Print Name	Signature	Date
<b>FINAL APPROVER</b>		
Matthew Tobolski, PhD, SE Print Name	 Signature	07/13/2015 Date

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## Document History

Rev.	Date	Reason for Revision	Revised by
0	7/13/2015	Initial Issue	James Linjun Yan, PhD, PE

## List of Effective Pages

Section	Pages	Revision
Body	269	0
Attachment A	15	0

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## Executive Summary

TWEI has been retained to develop a series of anchorage design details and calculations for seismic restraint of a series of freezer products by ThermoFisher Scientific. These anchor details are designed using an off-the-shelf system provided by QuakeHOLD! Industrial for use with systems in California or other seismic regions.

A total of 15 different freezers (as defined by cut sheets in Appendix A) were covered by this study. For each freezer, a kit for seismic restraint (per TWEI drawing 2015-0932-DD-001) will be provided, including the following essential components:

- A frame of unistrut installed against the freezer's base on all four sides to resist seismic shear.
- A strap with pretension force wrapping around the freezer from side to side, which is tied down to the unistrut frame to resist the unit overturning due to seismic force.
- Post-installed anchors (Powers Power-Stud+ SD2 & SD4 anchors per ICC-ES ESR 2502) for the connection of the unistrut frame to supporting concrete slab by others, to resist seismic shear and uplift force due to overturning.

A systematical check of each system was performed to ensure a complete load path can be established to provide the expected seismic restraint mechanism. The detailed calculations are documented in this report and the main design considerations are summarized below:

- Code standards: IBC 2012 and ASCE 7-10.
- Design seismic force for each freezer is developed per ASCE 7-10 Chapter 13 for nonstructural components, with the following seismic parameters:
  - Component amplification factor,  $a_p = 2.5$
  - Component response modification factor,  $R_p = 6$
  - Component important factor,  $I_p = 1.0$  or  $1.5$
  - Component attachment height within building,  $z/h = 0$  or  $1$
- Consideration of Center Gravity (C.G.):
  - Maximum height of C.G. is  $2/3$  of overall unit height.
  - Maximum 10% eccentricity of C.G. each horizontal direction.
- The supporting structure provided by others is a normal weight concrete slab of minimum 6in thickness and minimum compression strength,  $f'_c = 4000$ psi.
- Maximum site demands (in terms of short period spectral acceleration,  $S_{DS}$ ) considered for each freezer of different installation heights are summarized in the table on next page.

Please note that this study is only limited to ensure structural adequacy of the seismic restraint system provided for each freezer, which does not include structural integrity check of freezer itself.

CONFIGURATION NO.	UNIT NAME	UNIT BASE			CASTER	MAX. OPERATION WEIGHT	MAX. HEIGHT OF CENTER OF GRAVITY	MAX. S <sub>DS</sub>				REFERENCED DRAWING
		HEIGHT H	WIDTH B	DEPTH D	OUT-TO-OUT DEPTH D <sub>c</sub>			I <sub>p</sub> =1		I <sub>p</sub> =1.5		
		in	in	in	in			z/h=0	z/h=1	z/h=0	z/h=1	
		g	g	g	g							
1	LRF12	73.62	24.00	26.20	21.30	550	49.08	2.50	2.50	2.50	2.00	S101
2	LRF23	79.23	28.00	33.00	26.50	800	52.82	2.50	2.05	2.30	1.35	S102
3	LRF30	79.23	34.00	33.00	26.50	1000	52.82	2.50	1.65	1.80	1.10	S103
4A	LRF45	79.23	56.50	31.50	23.20	1100	52.82	2.50	1.50	1.65	1.00	S104A
4B	LRF50	79.23	56.50	33.00	26.50	1250	52.82	2.20	1.30	1.45	0.85	S104B
5	LRF75	79.23	85.00	33.00	26.50	1600	52.82	1.70	1.00	1.15	0.65	S105A&B
6	ULT13	77.90	33.30	29.50	24.30	1350	51.93	2.00	1.20	1.35	0.80	S106
7	ULT300	78.00	23.00	35.90	30.30	1300	52.00	2.10	1.25	1.40	0.85	S107
8	ULT400	78.00	28.40	35.90	30.30	1600	52.00	1.70	1.00	1.15	0.65	S108
9A	ULT500	78.00	34.00	35.90	30.30	1850	52.00	1.45	0.85	0.95	0.55	S109A
9B	ULT17	77.90	33.30	35.75	30.50	1500	51.93	1.80	1.10	1.20	0.70	S109B
10A	ULT600	78.00	39.60	35.90	30.30	2100	52.00	1.30	0.75	0.85	0.50	S110A
10B	ULT23	77.90	40.70	35.75	30.50	1650	51.93	1.65	1.00	1.10	0.65	S110B
11A	ULT700	78.00	45.30	35.90	30.30	2500	52.00	1.10	0.65	0.70	0.40	S111A
11B	ULT28	77.90	46.70	35.75	30.50	1800	51.93	1.50	0.90	1.00	0.60	S111B

S<sub>DS</sub>: SHORT PERIOD SPECTRAL ACCELERATION.  
z: HEIGHT IN BUILDING OF UNIT INSTALLATION WITH RESPECT TO THE BUILDING BASE.  
h: AVERAGE ROOF HEIGHT OF BUILDING WITH RESPECT TO THE BUILDING BASE.  
I<sub>p</sub>= 1.5, SHOWN IN THIS TABLE FOR ANCHORAGE DESIGN ONLY, NOT FOR CONSIDERATION OF SPECIAL SEISMIC CERTIFICATION.

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## **Section 1. Seismic Restraint Design of Freezers for $I_p = 1$ & $z/h=0$**

## Summary of Anchor Force


Configuration	Unit Name	Demand $S_{ps}$	Case 1		Case 2		Case 3	Case 4	Strap Pretension Force <b>lbf</b>
			$T_{u1}$ (lbf)	$V_{u2}$ (lbf)	$T_{u2,prying}$	$T_{u3}$ (lbf)			
1	LRF 12	2.50	1810.82	515.63	2300.48	2122.94	257.81	798.26	
2	LRF 23	2.50	2456.36	750.00	3346.15	2694.61	375.00	1004.37	
3	LRF 30	2.50	2541.23	937.50	4182.69	3368.27	468.75	1255.47	
4a	LRF 45	2.50	1664.60	1031.25	4600.96	4224.20	515.63	1591.18	
4b	LRF 50	2.20	1610.01	1031.25	4600.96	3653.43	515.63	1329.35	
5	LRF 75	1.70	868.62	1020.00	4550.77	3488.34	510.00	1189.61	
6	ULT 13	2.00	2652.42	1012.50	4517.31	3795.82	506.25	1361.91	
7	ULT 300	2.10	3898.93	1023.75	4567.50	3093.17	511.88	1405.98	
8	ULT 400	1.70	3053.20	1020.00	4550.77	2975.54	510.00	1001.69	
9a	ULT 500	1.45	2396.96	1005.94	4488.03	2839.63	502.97	873.44	
9b	ULT 17	1.80	2597.43	1012.50	4517.31	2958.76	506.25	994.59	
10a	ULT 600	1.30	1975.54	1023.75	4567.50	2814.14	511.88	813.14	
10b	ULT 23	1.65	2065.68	1020.94	4554.95	2935.31	510.47	954.77	
11a	ULT 700	1.10	1548.56	1031.25	4600.96	2700.60	515.63	684.94	
11b	ULT 28	1.50	1694.34	1012.50	4517.31	2853.81	506.25	889.63	
<b>Max</b>			<b>3898.93</b>	<b>1031.25</b>	<b>4600.96</b>	<b>4224.20</b>	<b>515.63</b>	<b>1591.18</b>	

$z/h =$	0
$I_p =$	1

Note:

- Case 1 Tension load to middle anchor group due to side to side seismic loading
- Case 2 Shear load & tension load by prying action to end anchor due to side to side seismic loading
- Case 3 Tension load to middle anchor group due to front to back seismic loading
- Case 4 Shear loading to end anchor due to front and back seismic loading





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 12  
**Configuratoin No.:** 1

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers


**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	550	lbf	per unit cut sheet
Unit width	$B$	24	in	per unit cut sheet
Unit depth	$D$	26.2	in	per unit cut sheet
Unit height	$H$	73.62	in	per unit cut sheet
Caster out to out distance	$D_c$	21.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	26.63	in	$S_{ss} = B+1''+1\ 5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	22.93	in	$S_{fb} = D_c+1\ 5/8''$
CG location	$H_{cg}$	49.08	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	2.62	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{bs}$	2.50		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	229.17	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2200.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	412.50	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	412.50	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



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- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

(1.2 + 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + αE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + αE<sub>y</sub>

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**  
**4.1 Side to Side Direction**


Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	20245.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	2400.75	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{SS})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	17844.75	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	<b>F<sub>SS</sub></b>	670.23	lbf	$F_{SS} = M_{R,strap} / S_{SS}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	20245.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	1945.35	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{fb})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	18300.15	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	<b>F<sub>fb</sub></b>	798.26	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	<b>T<sub>a</sub></b>	1500	lbf	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	<b>F<sub>max</sub></b>	798.26	lbf	$F_{max} = \max(F_{SS}, F_{fb})$
Demand capacity ratio	<b>DCR</b>	0.38	O.K.	DCR = F <sub>max</sub> / (1.4T <sub>a</sub> ), O.K. if DCR < 1.



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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unistrut allowable bending capacity for any dir. bending Modified bending capacity for HS/T Section Max spacing of anchors next to strap Max distance of end anchors to the end of unistrut	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
	$S_{anchor}$	4	in	
	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	670.23	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.11	O.K.	DCR = $M_{u1} / \phi M_n$

### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	798.26	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$



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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unistrut on front and back sides resisting seismic shear by sideway bending				
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	495.00	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	25.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.08	O.K.	$DCR = M_{u2} / \phi M_n$

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**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	1810.82	lb	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	515.63	lb	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	2300.48	lb	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	2122.94	lb	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	257.81	lb	$V_{u4} = \Omega * F_p / 4$
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## Sesimic Restraints of Freezers


**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	800	lbf	per unit cut sheet
Unit width	$B$	28	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	30.63	in	$S_{ss} = B + 1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.80	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{bs}$	2.50		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	333.33	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	3200.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	600.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	600.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

(1.2 + 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + αE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + αE<sub>y</sub>

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**

**4.1 Side to Side Direction**


Overturning moment due to seismic force	M <sub>OT</sub>	31692.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	M <sub>R,grav</sub>	4004.00	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{sy} / 2 - e_{ss})$
Resisting moment by strap	M <sub>R,strap</sub>	27688.00	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	F <sub>ss</sub>	904.10	lb	$F_{ss} = M_{R,strap} / S_{sb}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	M <sub>OT</sub>	31692.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	M <sub>R,grav</sub>	3444.00	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{sy} / 2 - e_{fb})$
Resisting moment by strap	M <sub>R,strap</sub>	28248.00	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	F <sub>fb</sub>	1004.37	lb	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	T <sub>a</sub>	1500	lb	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	1004.37	lb	$F_{max} = \max(F_{sv}, F_{fb})$
Demand capacity ratio	DCR	0.48	O.K.	DCR = F <sub>max</sub> / (1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 23  
**Configuratoin No.:** 2

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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### Seismic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

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#### 5. Check of Unistrut

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#### 5.1 Side to Side Direction

**Unistrut on one side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	904.10	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	per unistrut catalog page 56 for P1000 beam.
Capacity reduction factor	$\phi$	1		$\phi M_n = \phi * 1.4 * M_n$
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	DCR = $M_{u1} / \phi M_n$
Demand capacity ratio	DCR	0.15	O.K.	


**Unistrut on the other side resisting seismic shear by anchor shear**

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#### 5.2 Front to Back Direction

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	1004.37	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	per unistrut catalog page 56 for P1000 beam.
Capacity reduction factor	$\phi$	1		$\phi M_n = \phi * 1.4 * M_n$
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	DCR = $M_{u1} / \phi M_n$
Demand capacity ratio	DCR	0.17	O.K.	



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**Project Name:** Freezers (Thermo Fisher Scientific)  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unistrut on front and back sides resisting seismic shear by sideway bending				
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	840.00	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	29.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.96		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5791.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.15	O.K.	$DCR = M_{u2} / \phi M_n$

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**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	2456.36	lb	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	750.00	lb	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	3346.15	lb	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**


**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	2694.61	lb	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	375.00	lb	$V_{u4} = \Omega * F_p / 4$
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**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 30  
**Configuratoin No.:** 3

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers


**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1000	lbf	per unit cut sheet
Unit width	$B$	34	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	36.63	in	$S_{ss} = B + 1" + 1/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c + 1/8"$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{bs}$	2.5		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	416.67	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4000.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	750.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	750.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



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### Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

(1.2 + 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 Section 12.4.3.2 basic load comb 5 for x dir

(1.2 + 0.2S<sub>DS</sub>)D + αE<sub>y</sub>  
 Section 12.4.3.2 basic load comb 5 for y dir

(0.9 - 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 Section 12.4.3.2 basic load comb 7 for x dir

(0.9 - 0.2S<sub>DS</sub>)D + αE<sub>y</sub>  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**

**4.1 Side to Side Direction**


Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	39615.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	5965.00	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{sy} / 2 - e_{ss})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	33650.00	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	<b>F<sub>SS</sub></b>	918.77	lb	$F_{SS} = M_{R,strap} / S_{sb}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	39615.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	4305.00	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{yb} / 2 - e_{fb})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	35310.00	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	<b>F<sub>fb</sub></b>	1255.47	lb	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	<b>T<sub>a</sub></b>	1500	lb	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	<b>F<sub>max</sub></b>	1255.47	lb	$F_{max} = \max(F_{sv}, F_{fb})$
Demand capacity ratio	<b>DCR</b>	0.60	O.K.	DCR = F <sub>max</sub> / (1.4T <sub>a</sub> ), O.K. if DCR < 1.



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**Reviewer:** MT  
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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	918.77	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.15	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	1255.47	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.21	O.K.	DCR = $M_{u1} / \phi M_n$



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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1275.00	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	35.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.94		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5671.30	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.22	O.K.	$DCR = M_{u2} / \phi M_n$

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**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	2541.23	lb	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	937.50	lb	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4182.69	lb	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	3368.27	lb	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	468.75	lb	$V_{u4} = \Omega * F_p / 4$
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**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 45  
**Configuratoin No.:** 4a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers


**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1100	lbf	per unit cut sheet
Unit width	$B$	56.5	in	per unit cut sheet
Unit depth	$D$	31.5	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	23.2	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	59.13	in	$S_{ss} = B+1''+1\ 5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	24.83	in	$S_{fb} = D_c+1\ 5/8''$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	5.65	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.15	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{bs}$	2.50		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	458.33	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4400.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	825.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**

**4.1 Side to Side Direction**


Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	10521.50	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	33055.00	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	559.07	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	4075.50	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	39501.00	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1591.18	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1591.18	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	<b>DCR</b>	0.76	O.K.	$DCR = F_{max} / (1.4 T_a)$ , O.K. if $DCR < 1$ .



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**Originator:** JY  
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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	559.07	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.09	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	1591.18	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.26	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 45  
**Configuratoin No.:** 4a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unistrut on front and back sides resisting seismic shear by sideway bending				
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	2330.63	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	57.50	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.39	O.K.	$DCR = M_{u2} / \phi M_n$

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**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	1664.60	lb	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1031.25	lb	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4600.96	lb	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**


**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	4224.20	lb	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	515.63	lb	$V_{u4} = \Omega * F_p / 4$
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**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratoin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers


**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1250	lbf	per unit cut sheet
Unit width	$B$	56.5	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	59.13	in	$S_{ss} = B+1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	5.65	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{bs}$	2.20		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	458.33	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4400.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	825.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratoin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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### Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**

**4.1 Side to Side Direction**


Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43576.50	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	13749.69	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{Sx} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	29826.81	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{SS}$	504.47	lb	$F_{SS} = M_{R,strap} / S_{SS}$

**4.2 Front to Back Direction**

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43576.50	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	6188.44	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{Sx} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	37388.06	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1329.35	lb	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Description	Variable	Value	Units	Equation / Reference
Tested allowable capacity of strap	$T_a$	1500	lb	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1329.35	lb	$F_{max} = \max(F_{SS}, F_{fb})$
Demand capacity ratio	DCR	0.63	O.K.	$DCR = F_{max} / (1.4 T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	504.47	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	per unistrut catalog page 56 for P1000 beam.
Demand capacity ratio	DCR	0.08	O.K.	$\phi M_n = f * 1.4 * M_a$ DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	1329.35	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	per unistrut catalog page 56 for P1000 beam.
Demand capacity ratio	DCR	0.22	O.K.	$\phi M_n = f * 1.4 * M_a$ DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratoin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unistrut on front and back sides resisting seismic shear by sideway bending				
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	2330.63	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	57.50	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.39	O.K.	$DCR = M_{u2} / \phi M_n$

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**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	1610.01	lb	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1031.25	lb	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4600.96	lb	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	3653.43	lb	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	515.63	lb	$V_{u4} = \Omega * F_p / 4$
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**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 75  
**Configuratoin No.:** 5

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	1600	lbf	per unit cut sheet
Unit width	$B$	85	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	87.63	in	$S_{ss} = B + 1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	8.50	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{bs}$	1.70		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	453.33	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4352.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	816.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	816.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 75  
**Configuratoin No.:** 5

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

(1.2 + 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 Section 12.4.3.2 basic load comb 5 for x dir

(1.2 + 0.2S<sub>DS</sub>)D + αE<sub>y</sub>  
 Section 12.4.3.2 basic load comb 5 for y dir

(0.9 - 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 Section 12.4.3.2 basic load comb 7 for x dir

(0.9 - 0.2S<sub>DS</sub>)D + αE<sub>y</sub>  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**

**4.1 Side to Side Direction**


Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	43101.12	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	31640.00	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{sy} / 2 - e_{ss})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	11461.12	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	<b>F<sub>SS</sub></b>	130.80	lb	$F_{SS} = M_{R,strap} / S_{sb}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	43101.12	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	9643.20	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{yb} / 2 - e_{fb})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	33457.92	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	<b>F<sub>fb</sub></b>	1189.61	lb	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	<b>T<sub>a</sub></b>	1500	lb	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	<b>F<sub>max</sub></b>	1189.61	lb	$F_{max} = \max(F_{sv}, F_{fb})$
Demand capacity ratio	<b>DCR</b>	0.57	O.K.	DCR = F <sub>max</sub> / (1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 75  
**Configuratin No.:** 5

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	130.80	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	per unistrut catalog page 56 for P1000 beam.
Demand capacity ratio	DCR	0.02	O.K.	$\phi M_n = f * 1.4 * M_a$ DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	1189.61	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	per unistrut catalog page 56 for P1000 beam.
Demand capacity ratio	DCR	0.20	O.K.	$\phi M_n = f * 1.4 * M_a$ DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 75  
**Configuratoin No.:** 5

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unistrut on front and back sides resisting seismic shear by sideway bending				
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	3468.00	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	86.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.58	O.K.	$DCR = M_{u2} / \phi M_n$

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**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	868.62	lb	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1020.00	lb	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4550.77	lb	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**


**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	3488.34	lb	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	510.00	lb	$V_{u4} = \Omega * F_p / 4$
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**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 13  
**Configuratoin No.:** 6

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers


**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1350	lbf	per unit cut sheet
Unit width	$B$	33.3	in	per unit cut sheet
Unit depth	$D$	29.5	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	24.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	35.93	in	$S_{ss} = B + 1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	25.93	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	51.93	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.33	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	2.95	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{bs}$	2.00		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	450.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4320.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	810.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



**TW**  
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Tobolski Watkins Engineering, Inc.

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 13  
**Configuratin No.:** 6

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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### Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

$(1.2 + 0.2S_{DS})D + \Omega E_x$	
$(1.2 + 0.2S_{DS})D + \Omega E_y$	
$(0.9 - 0.2S_{DS})D + \Omega E_x$	
$(0.9 - 0.2S_{DS})D + \Omega E_y$	

**4. Check of Strap**

**4.1 Side to Side Direction**


Overturning moment due to seismic force	$M_{OT}$	42066.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	9876.94	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_y / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	32189.06	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	896.01	lb	$F_{ss} = M_{R,strap} / S_{ss}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	$M_{OT}$	42066.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	6758.44	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_y / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	35307.56	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1361.91	lb	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	$T_a$	1500	lb	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1361.91	lb	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.65	O.K.	DCR = $F_{max} / (1.4 T_a)$ , O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 13  
**Configuratoin No.:** 6

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	896.01	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	$\phi$	1		
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	per unistrut catalog page 56 for P1000 beam.
Demand capacity ratio	DCR	0.15	O.K.	$\phi M_n = \phi * 1.4 * M_a$ DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	1361.91	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	$\phi$	1		
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	per unistrut catalog page 56 for P1000 beam.
Demand capacity ratio	DCR	0.23	O.K.	$\phi M_n = \phi * 1.4 * M_a$ DCR = $M_{u1} / \phi M_n$

**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1348.65	lbf-in	$M_{u2} = F_{ps} / 2 * 0.1 * B$
Unbraced length	L	34.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_a$
Demand capacity ratio	DCR	0.23	O.K.	DCR = $M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

 <p><b>Tobolski Watkins Engineering, Inc.</b></p>	<p><b>Project Name:</b> Freezers (Thermo Fisher Scientific)  <b>Project No.:</b> 0932  <b>Item:</b> ULT 13  <b>Configuratoin No.:</b> 6</p>	<p><b>Cal. No.:</b> 2015-0932-DC-001, r0  <b>Originator:</b> JY  <b>Reviewer:</b> MT  <b>Date:</b> 4/10/2015</p>		
<h3>Sesimic Restraints of Freezers</h3>				
<p><b>Notes:</b></p> <ol style="list-style-type: none"> <li>Seismic force is determined per Section 13.3 of ASCE 7.</li> <li>Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.</li> <li>In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.</li> </ol>				
Description	Variable	Value	Units	Equation / Reference

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{SS}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u2}$   lbf  $V_{u2} = \Omega * F_p / 2$   
 Ultimate prying tension force on anchor due to shear  $T_{u2,prying}$   lbf  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 300  
**Configuratoin No.:** 7

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers


**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1300	lbf	per unit cut sheet
Unit width	$B$	23	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	25.63	in	$S_{ss} = B + 1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	52.00	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.30	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{bs}$	2.10		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	455.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4368.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	819.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	819.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42588.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	6559.80	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	36028.20	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	1405.98	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

**4. Check of Strap**

**4.1 Side to Side Direction**


Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42588.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	7720.44	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	34867.56	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1092.17	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.2 Front to Back Direction**

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42588.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	7720.44	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	34867.56	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1092.17	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Description	Variable	Value	Units	Equation / Reference
Tested allowable capacity of strap	$T_a$	1500	lbf	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1405.98	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.67	O.K.	$DCR = F_{max} / (1.4 T_a)$ , O.K. if $DCR < 1$ .



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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	1405.98	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.23	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	1092.17	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.18	O.K.	DCR = $M_{u1} / \phi M_n$



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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	941.85	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	24.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.16	O.K.	$DCR = M_{u2} / \phi M_n$

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**Unistrut on front and back sides resisting seismic shear by sideway bending**

Ultimate tension force on anchor group due to overturning  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$  3898.93 lbf

**End anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate shear force on anchor  $V_{u2} = \Omega * F_p / 2$  1023.75 lbf

Ultimate prying tension force on anchor due to shear  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$  4567.50 lbf

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**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$  3093.17 lbf

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4} = \Omega * F_p / 4$  511.88 lbf

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$  3898.93 lbf

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u2} = \Omega * F_p / 2$  1023.75 lbf

Ultimate prying tension force on anchor due to shear  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$  4567.50 lbf

**6.2 Front to Back Direction**


**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$  3093.17 lbf

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4} = \Omega * F_p / 4$  511.88 lbf





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 400  
**Configuratoin No.:** 8

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers


**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1600	lbf	per unit cut sheet
Unit width	$B$	28.4	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	31.03	in	$S_{ss} = B + 1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	52.00	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.84	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{bs}$	1.70		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	453.33	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4352.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	816.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	816.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

$(1.2 + 0.2S_{DS})D + \Omega E_x$
$(1.2 + 0.2S_{DS})D + \Omega E_y$
$(0.9 - 0.2S_{DS})D + \Omega E_x$
$(0.9 - 0.2S_{DS})D + \Omega E_y$

**4. Check of Strap**

**4.1 Side to Side Direction**


Overturning moment due to seismic force	$M_{OT}$	42432.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	11354.56	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{sy} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	31077.44	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	1001.69	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	$M_{OT}$	42432.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	11085.76	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{yb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	31346.24	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	981.87	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1001.69	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	<b>DCR</b>	0.48	O.K.	$DCR = F_{max} / (1.4 T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 400  
**Configuratin No.:** 8

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	1001.69	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.17	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	981.87	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u1} / \phi M_n$



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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1158.72	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	29.40	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.19	O.K.	$DCR = M_{u2} / \phi M_n$

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**Unistrut on front and back sides resisting seismic shear by sideway bending**

Ultimate tension force on anchor group due to overturning  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$

Ultimate tension force on anchor group due to overturning  $T_{u1} = 3053.20$  lbf

**End anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate shear force on anchor  $V_{u2} = \Omega * F_p / 2$

Ultimate prying tension force on anchor due to shear  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

Ultimate shear force on anchor  $V_{u2} = 1020.00$  lbf

Ultimate prying tension force on anchor due to shear  $T_{u2,prying} = 4550.77$  lbf

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**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**


Ultimate tension force on anchor group due to overturning  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{Rb} / 2) / 2$

Ultimate tension force on anchor group due to overturning  $T_{u3} = 2975.54$  lbf

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4} = \Omega * F_p / 4$

Ultimate shear force on anchor  $V_{u4} = 510.00$  lbf



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 500  
**Configuratoin No.:** 9a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1850	lbf	per unit cut sheet
Unit width	$B$	34	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	36.63	in	$S_{ss} = B+1''+1\ 5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c+1\ 5/8''$
CG location	$H_{cg}$	52.00	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue


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**1. Unit Basic Information**

Short period spectral response acceleration	$S_{bs}$	1.45		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	447.08	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4292.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	804.75	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	804.75	lbf	$\min(F_{p,max}, \max(F_{p,min}))$

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**2. Seismic Force Calculation**



**Tobolski Watkins Engineering, Inc.**

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**

**4.1 Side to Side Direction**


Overturning moment due to seismic force	$M_{OT}$	41847.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	16828.76	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{SS})$
Resisting moment by strap	$M_{R,strap}$	25018.24	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{SS}$	683.09	lb	$F_{SS} = M_{R,strap} / S_{SS}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	$M_{OT}$	41847.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	13962.37	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	27884.63	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	873.44	lb	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	$T_a$	1500	lb	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	873.44	lb	$F_{max} = \max(F_{SS}, F_{fb})$
Demand capacity ratio	$DCR$	0.42	O.K.	$DCR = F_{max} / (1.4 T_a)$ , O.K. if $DCR < 1$ .



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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	683.09	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.11	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	873.44	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.14	O.K.	DCR = $M_{u1} / \phi M_n$

 <p><b>Tobolski Watkins Engineering, Inc.</b></p>	<p><b>Project Name:</b> Freezers (Thermo Fisher Scientific)  <b>Project No.:</b> 0932  <b>Item:</b> ULT 500  <b>Configuratoin No.:</b> 9a</p>	<p><b>Cal. No.:</b> 2015-0932-DC-001, r0  <b>Originator:</b> JY  <b>Reviewer:</b> MT  <b>Date:</b> 4/10/2015</p>	
<h2>Sesimic Restraints of Freezers</h2>			
<p><b>Notes:</b></p> <ol style="list-style-type: none"> <li>Seismic force is determined per Section 13.3 of ASCE 7.</li> <li>Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.</li> <li>In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.</li> </ol>			
Description	Variable	Value	Equation / Reference

**Unistrut on front and back sides resisting seismic shear by sideway bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1368.08	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	35.00	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99	per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.23	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	2396.96	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1005.94	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4488.03	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**


**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	2839.63	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	502.97	$V_{u4} = \Omega * F_p / 4$
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**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 17  
**Configuratoin No.:** 9b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**


- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1500	lbf	per unit cut sheet
Unit width	$B$	33.3	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	35.93	in	$S_{ss} = B + 1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	51.93	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.33	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{bs}$	1.80		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	450.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4320.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	810.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



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**Originator:** JY  
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### Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

(1.2 + 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + αE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + αE<sub>y</sub>

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**

**4.1 Side to Side Direction**


Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	42066.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	11852.33	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{sy} / 2 - e_{ss})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	30213.68	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	<b>F<sub>SS</sub></b>	841.02	lbf	$F_{SS} = M_{R,strap} / S_{ss}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	42066.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	10114.88	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{yb} / 2 - e_{fb})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	31951.13	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	<b>F<sub>fb</sub></b>	994.59	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	<b>T<sub>a</sub></b>	1500	lbf	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	<b>F<sub>max</sub></b>	994.59	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	<b>DCR</b>	0.47	O.K.	$DCR = F_{max} / (1.4 T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

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**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	841.02	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.14	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	994.59	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 17  
**Configuratoin No.:** 9b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unistrut on front and back sides resisting seismic shear by sideway bending				
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1348.65	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	34.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.23	O.K.	$DCR = M_{u2} / \phi M_n$

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**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	2597.43	lb	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1012.50	lb	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4517.31	lb	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	2958.76	lb	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	506.25	lb	$V_{u4} = \Omega * F_p / 4$
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**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
**Configuratoin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers


**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	21.00	lbf	per unit cut sheet
Unit width	$B$	39.6	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	42.23	in	$S_{ss} = B + 1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	52.00	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.96	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{bs}$	1.30		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	455.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4368.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	819.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	819.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
**Configuratoin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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### Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

(1.2 + 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + αE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + αE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + αE<sub>y</sub>

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**  
**4.1 Side to Side Direction**


Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	42588.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	23052.96	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{sy} / 2 - e_{ss})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	19535.04	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	<b>F<sub>ss</sub></b>	462.64	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	<b>M<sub>OT</sub></b>	42588.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	<b>M<sub>R,grav</sub></b>	16628.64	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{yb} / 2 - e_{fb})$
Resisting moment by strap	<b>M<sub>R,strap</sub></b>	25959.36	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	<b>F<sub>fb</sub></b>	813.14	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	<b>T<sub>a</sub></b>	1500	lbf	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	<b>F<sub>max</sub></b>	813.14	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	<b>DCR</b>	0.39	O.K.	DCR = F <sub>max</sub> / (1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
**Configuratin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	462.64	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.08	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	813.14	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
**Configuratoin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1621.62	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	40.60	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.27	O.K.	$DCR = M_{u2} / \phi M_n$

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**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	1975.54	lb	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1023.75	lb	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4567.50	lb	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	2814.14	lb	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{Rb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	511.88	lb	$V_{u4} = \Omega * F_p / 4$
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**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 23  
**Configuratoin No.:** 10b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

Tobolski Watkins Engineering, Inc.

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	1.650	lbf	per unit cut sheet
Unit width	$B$	40.7	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	43.33	in	$S_{ss} = B + 1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	51.93	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.07	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.65		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	453.75	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4356.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	816.75	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	816.75	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 23  
**Configuratoin No.:** 10b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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### Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

$(1.2 + 0.2S_{DS})D + \Omega E_x$	
$(1.2 + 0.2S_{DS})D + \Omega E_y$	
$(0.9 - 0.2S_{DS})D + \Omega E_x$	
$(0.9 - 0.2S_{DS})D + \Omega E_y$	

**4. Check of Strap**  
**4.1 Side to Side Direction**


Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42416.55	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	16545.75	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{sy} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	25870.80	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	597.13	lb	$F_{ss} = M_{R,strap} / S_{ss}$

**4.2 Front to Back Direction**

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42416.55	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	11744.49	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{yb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	30672.06	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	954.77	lb	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Description	Variable	Value	Units	Equation / Reference
Tested allowable capacity of strap	$T_a$	1500	lb	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	954.77	lb	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.45	O.K.	$DCR = F_{max} / (1.4 T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 23  
**Configuratin No.:** 10b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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### Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	597.13	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.10	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	954.77	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 23  
**Configuratoin No.:** 10b

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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1662.09	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	41.70	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.28	O.K.	$DCR = M_{u2} / \phi M_n$

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**Unistrut on front and back sides resisting seismic shear by sideway bending**

Ultimate tension force on anchor group due to overturning  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$

Ultimate shear force on anchor  $V_{u2} = \Omega * F_p / 2$

Ultimate prying tension force on anchor due to shear  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

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**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1} = 2065.68$  lbf

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u2} = 1020.94$  lbf

Ultimate prying tension force on anchor due to shear  $T_{u2,prying} = 4554.95$  lbf

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**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3} = 2935.31$  lbf

$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{Rb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4} = 510.47$  lbf

$V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	2500	lbf	per unit cut sheet
Unit width	$B$	45.3	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	47.93	in	$S_{ss} = B+1" +1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c+1 5/8"$
CG location	$H_{cg}$	52.00	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.53	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.10		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	458.33	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4400.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	825.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Project Name:** Freezers (Thermo Fisher Scientific) **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932 **Originator:** JY  
**Item:** ULT 700 **Reviewer:** MT  
**Configuratin No.:** 11a **Date:** 4/10/2015

### Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$  Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$  Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$  Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$  Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42900.00	lb-ft-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	33035.25	lb-ft-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{sf} / 2 - e_{sf})$
Resisting moment by strap	$M_{R,strap}$	9864.75	lb-ft-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{SS}$	205.84	lbf	$F_{SS} = M_{R,strap} / S_{SS}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42900.00	lb-ft-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	21033.25	lb-ft-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	21866.75	lb-ft-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	684.94	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	684.94	lbf	$F_{max} = \max(F_{SS}, F_{fb})$
Demand capacity ratio	DCR	0.33	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 700  
**Configuratin No.:** 11a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	205.84	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.03	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	684.94	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.11	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
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**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1868.63	lb-ft	$M_{u2} = F_d/2 * 0.1 * B$
Unbraced length	L	46.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.31	O.K.	$DCR = M_{u2} / \phi M_n$

### Unistrut on front and back sides resisting seismic shear by sideways bending

### 6. Calculation of Post Installed Anchor Force

#### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u2}$   lbf  $V_{u2} = \Omega * F_p / 2$   
 Ultimate prying tension force on anchor due to shear  $T_{u2,prying}$   lbf  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

#### 6.2 Front to Back Direction


#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 28  
**Configuratoin No.:** 11b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1800	lbf	per unit cut sheet
Unit width	$B$	46.7	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	49.33	in	$S_{ss} = B + 1" + 1 5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c + 1 5/8"$
CG location	$H_{cg}$	51.93	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.67	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 1. Unit Basic Information

Description	Variable	Value	Units	Equation / Reference
Short period spectral response acceleration	$S_{bs}$	1.50		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	450.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4320.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	810.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$

### 2. Seismic Force Calculation

Description	Variable	Value	Units	Equation / Reference
Short period spectral response acceleration	$S_{bs}$	1.50		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	450.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4320.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	810.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 28  
**Configuratoin No.:** 11b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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### Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**3. Load Combinations**

Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

**4. Check of Strap**

**4.1 Side to Side Direction**


Overturning moment due to seismic force	$M_{OT}$	42066.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	21591.90	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	20474.10	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	415.09	lb	$F_{ss} = M_{R,strap} / S_{ss}$

**4.2 Front to Back Direction**

Overturning moment due to seismic force	$M_{OT}$	42066.00	lb-ft	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	13486.50	lb-ft	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{S_y} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	28579.50	lb-ft	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	889.63	lb	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

**4.3 Check of Strap**

Tested allowable capacity of strap	$T_a$	1500	lb	per information of Quakhold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	889.63	lb	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.42	O.K.	DCR = $F_{max} / (1.4 T_a)$ , O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

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## Sesimic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	415.09	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.07	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	889.63	lbf-in	$M_{u1} = F_{ts} * S_{anchor} / 4$
Unbraced length	$L$	4	in	
Capacity reduction factor	$\phi$	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.15	O.K.	DCR = $M_{u1} / \phi M_n$

 <p><b>Tobolski Watkins Engineering, Inc.</b></p>	<p><b>Project Name:</b> Freezers (Thermo Fisher Scientific)  <b>Project No.:</b> 0932  <b>Item:</b> ULT 28  <b>Configuratoin No.:</b> 11b</p>	<p><b>Cal. No.:</b> 2015-0932-DC-001, r0  <b>Originator:</b> JY  <b>Reviewer:</b> MT  <b>Date:</b> 4/10/2015</p>	
<h2>Sesimic Restraints of Freezers</h2>			
<p><b>Notes:</b></p> <ol style="list-style-type: none"> <li>Seismic force is determined per Section 13.3 of ASCE 7.</li> <li>Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post-installed anchors of max 4in spacing.</li> <li>In addition, for unistrut on the side, one post-installed anchor is provided at max 6.5in from each end.</li> </ol>			
Description	Variable	Value	Equation / Reference

**Unistrut on front and back sides resisting seismic shear by sideway bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1891.35	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	47.70	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99	per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	$\phi M_n = \phi * 1.4 * M_n$
Demand capacity ratio	DCR	0.32	DCR = $M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u1}$	1694.34	$T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{S5}$
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**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1012.50	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4517.31	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning	$T_{u3}$	2853.81	$T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$
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**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u4}$	506.25	$V_{u4} = \Omega * F_p / 4$
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## **Section 2. Seismic Restraint Design of Freezers for $I_p = 1$ & $z/h=1$**

## Summary of Anchor Force

Configuration	Unit Name	Demand $S_{ps}$	Case 1		Case 2		Case 3	Case 4	Strap Pretension Force
			$T_{u1}$ (lbf)	$V_{u2}$ (lbf)	$T_{u2,prying}$	$T_{u3}$ (lbf)			
1	LRF 12	2.50	3078.14	859.38	3834.13	3594.80	429.69	1387.01	
2	LRF 23	2.05	3375.55	1025.00	4573.08	3699.99	512.50	1389.99	
3	LRF 30	1.65	2742.42	1031.25	4600.96	3655.35	515.63	1331.27	
4a	LRF 45	1.50	1575.63	1031.25	4600.96	4142.11	515.63	1509.09	
4b	LRF 50	1.30	1491.09	1015.63	4531.25	3508.64	507.81	1219.78	
5	LRF 75	1.00	754.24	1000.00	4461.54	3327.50	500.00	1073.85	
6	ULT 13	1.20	2564.44	1012.50	4517.31	3712.40	506.25	1278.49	
7	ULT 300	1.25	3775.30	1015.63	4531.25	2981.06	507.81	1302.12	
8	ULT 400	1.00	2894.66	1000.00	4461.54	2823.58	500.00	883.38	
9a	ULT 500	0.85	2240.91	982.81	4384.86	2678.26	491.41	757.27	
9b	ULT 17	1.10	2566.11	1031.25	4600.96	2937.75	515.63	937.21	
10a	ULT 600	0.75	1784.72	984.38	4391.83	2596.34	492.19	672.30	
10b	ULT 23	1.00	2003.30	1031.25	4600.96	2885.27	515.63	884.73	
11a	ULT 700	0.65	1423.42	1015.63	4531.25	2562.50	507.81	577.38	
11b	ULT 28	0.90	1606.79	1012.50	4517.31	2769.84	506.25	805.67	
<b>Max</b>			<b>3775.30</b>	<b>1031.25</b>	<b>4600.96</b>	<b>4142.11</b>	<b>515.63</b>	<b>1509.09</b>	

$z/h =$	1
$I_p =$	1

Note:

- Case 1 Tension load to middle anchor group due to side to side seismic loading
- Case 2 Shear load & tension load by prying action to end anchor due to side to side seismic loading
- Case 3 Tension load to middle anchor group due to front to back seismic loading
- Case 4 Shear loading to end anchor due to front and back seismic loading



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 12      **Reviewer:** MT  
**Configuratin No.:** 1      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	550	lbf	per unit cut sheet
Unit width	$B$	24	in	per unit cut sheet
Unit depth	$D$	26.2	in	per unit cut sheet
Unit height	$H$	73.62	in	per unit cut sheet
Caster out to out distance	$D_c$	21.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	26.63	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	22.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{cg}$	49.08	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	2.62	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	2.50		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	687.50	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2200.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	412.50	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	687.50	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 12      **Reviewer:** MT  
**Configuratin No.:** 1      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- | Description | Variable | Value                             | Units | Equation / Reference                         |
|-------------|----------|-----------------------------------|-------|--|
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 5 for x dir |
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 5 for y dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 7 for x dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	33742.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	2400.75	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	31341.75	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	1177.15	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	33742.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	1945.35	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	31797.15	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1387.01	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1387.01	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.66	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 12      **Reviewer:** MT  
**Configuratin No.:** 1      **Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	1177.15	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.20	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1387.01	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.23	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
 Item: LRF 12  
**Configuratin No.:** 1

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	825.00	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	25.00	in	$L = B + 1'$
Capacity reduction factor	φ	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.14	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  3078.14 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	859.38	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	3834.13	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$  3594.80 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  429.69 lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 23  
**Configuratin No.:** 2

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	800	lbf	per unit cut sheet
Unit width	$B$	28	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	30.63	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{eg}$	52.82	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.80	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	2.05		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	820.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2624.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	492.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	820.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 23      **Reviewer:** MT  
**Configuratin No.:** 2      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- | Description | Variable | Value                             | Units | Equation / Reference                         |
|-------------|----------|-----------------------------------|-------|--|
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 5 for x dir |
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 5 for y dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 7 for x dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction


Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43312.40	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	4904.90	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	38407.50	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	1254.12	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43312.40	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	4218.90	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	39093.50	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1389.99	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1389.99	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.66	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 23      **Reviewer:** MT  
**Configuratin No.:** 2      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	1254.12	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.21	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1389.99	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.23	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 23  
**Configuratin No.:** 2

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1148.00	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	29.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.96		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5791.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.20	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1025.00	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4573.08	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 30  
**Configuratin No.:** 3

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1000	lbf	per unit cut sheet
Unit width	$B$	34	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	36.63	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.65		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	825.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2640.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	495.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 30      **Reviewer:** MT  
**Configuratin No.:** 3      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	8500.13	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	35076.38	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	957.72	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	6134.63	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	37441.88	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1331.27	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1331.27	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.63	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 30      **Reviewer:** MT  
**Configuratin No.:** 3      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	957.72	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1331.27	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.22	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 30  
**Configuratin No.:** 3

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1402.50	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	35.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.94		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5671.30	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.25	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1031.25	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4600.96	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{Sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 45  
**Configuratin No.:** 4a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1100	lbf	per unit cut sheet
Unit width	$B$	56.5	in	per unit cut sheet
Unit depth	$D$	31.5	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	23.2	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	59.13	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	24.83	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.82	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	5.65	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.15	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.50		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	825.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2640.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	495.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 45      **Reviewer:** MT  
**Configuratin No.:** 4a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	15782.25	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	27794.25	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	470.09	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	6113.25	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	37463.25	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1509.09	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1509.09	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.72	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 45      **Reviewer:** MT  
**Configuratin No.:** 4a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**5. Check of Unistrut**

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

**5.1 Side to Side Direction**

**Unistrut on one side resisting strap force by bending in vertical direction**


Ultimate moment in unistrut due to strap force	$M_{u1}$	470.09	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.08	O.K.	DCR = $M_{u1} / \phi M_n$

**Unistrut on the other side resisting seismic shear by anchor shear**

**5.2 Front to Back Direction**

**Unistrut on each side resisting strap force by bending in vertical direction**

Ultimate moment in unistrut due to strap force	$M_{u1}$	1509.09	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.25	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 45      **Reviewer:** MT  
**Configuratin No.:** 4a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	2330.63	lb-ft-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	57.50	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.39	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning       $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

1575.63      lbf

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor       $V_{u2} = \Omega * F_p / 2$

Ultimate prying tension force on anchor due to shear       $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

1031.25      lbf

4600.96      lbf

##### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)


Ultimate tension force on anchor group due to overturning       $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{Sb} / 2) / 2$

4142.11      lbf

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor       $V_{u4} = \Omega * F_p / 4$

515.63      lbf


 <b>Tobolski Watkins Engineering, Inc.</b>	<b>Project Name:</b> Freezers (Thermo Fisher Scientific) <b>Project No.:</b> 0932 <b>Item:</b> LRF 50 <b>Configuratin No.:</b> 4b	<b>Cal. No.:</b> 2015-0932-DC-001, r0 <b>Originator:</b> JY <b>Reviewer:</b> MT <b>Date:</b> 4/10/2015
	<h2>Sesimic Restraints of Freezers</h2>	
<b>Notes:</b> 1. Seismic force is determined per Section 13.3 of ASCE 7. 2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing. 3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.		

### 1. Unit Basic Information

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1250	lbf	per unit cut sheet
Unit width	$B$	56.5	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	59.13	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	5.65	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{ps}$	1.30		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	812.50	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2600.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	487.50	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	812.50	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42916.25	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	19130.00	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	23786.25	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	402.30	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

#### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42916.25	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	8610.00	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	34306.25	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	1219.78	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

#### 4.3 Check of Strap

Description	Variable	Value	Units	Equation / Reference
Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	1219.78	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.58	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 50      **Reviewer:** MT  
**Configuratin No.:** 4b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	402.30	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.07	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1219.78	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.20	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	2295.31	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	57.50	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.38	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1015.63	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4531.25	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 75  
**Configuratin No.:** 5

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1600	lbf	per unit cut sheet
Unit width	$B$	85	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	87.63	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{eg}$	52.82	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	8.50	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.00		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	800.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2560.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	480.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	800.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
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**Item:** LRF 75      **Reviewer:** MT  
**Configuratin No.:** 5      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42256.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	39550.00	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	2706.00	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	30.88	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42256.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	12054.00	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	30202.00	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1073.85	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1073.85	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.51	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
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**Configuratin No.:** 5      **Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	30.88	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.01	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1073.85	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.18	O.K.	DCR = $M_{u1} / \phi M_n$



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**Project Name:** Freezers (Thermo Fisher Scientific)  
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**Originator:** JY  
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**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	3400.00	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	86.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.57	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1000.00	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4461.54	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT.13  
**Configuratin No.:** 6

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1350	lbf	per unit cut sheet
Unit width	$B$	33.3	in	per unit cut sheet
Unit depth	$D$	29.5	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	24.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	35.93	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	25.93	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.33	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	2.95	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.20		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	810.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2592.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	486.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.13      **Reviewer:** MT  
**Configuration No.:** 6      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42066.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	13037.56	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	29028.44	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	808.03	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

#### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42066.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	8921.14	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	33144.86	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	1278.49	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)

#### 4.3 Check of Strap

Description	Variable	Value	Units	Equation / Reference
Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	1278.49	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.61	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.





**Tobolski Watkins Engineering, Inc.**

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**Item:** ULT.13      **Reviewer:** MT  
**Configuratin No.:** 6      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	808.03	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1278.49	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.21	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

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**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.13      **Reviewer:** MT  
**Configuratin No.:** 6      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1348.65	lb-ft-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	34.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.23	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning       $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor       $V_{u2} = \Omega * F_p / 2$   
 Ultimate prying tension force on anchor due to shear       $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


##### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning       $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor       $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
 Item: ULT 300  
**Configuratin No.:** 7

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 1. Unit Basic Information

Unit weight	<b>W<sub>p</sub></b>	1300		per unit cut sheet
Unit width	<b>B</b>	23	in	per unit cut sheet
Unit depth	<b>D</b>	35.9	in	per unit cut sheet
Unit height	<b>H</b>	78	in	per unit cut sheet
Caster out to out distance	<b>D<sub>c</sub></b>	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	<b>S<sub>ss</sub></b>	25.63	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	<b>S<sub>fb</sub></b>	31.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	<b>H<sub>eg</sub></b>	52.00	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	<b>e<sub>ss</sub></b>	2.30	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	<b>e<sub>fb</sub></b>	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	<b>h<sub>unistrut</sub></b>	3.63	in	per unistrut catalogue
Depth of unistrut	<b>d<sub>unistrut</sub></b>	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	<b>S<sub>ps</sub></b>	1.25		normalized height
Average roof height of structure	<b>h</b>	1	in	normalized height
Height in structure of component attachment	<b>z</b>	1	in	
Component repsonse amplification factor	<b>a<sub>p</sub></b>	2.5	in	
Component repsonse modification factor	<b>R<sub>p</sub></b>	6	in	
Overstrength factor	<b>Ω</b>	2.5	in	
Component important factor	<b>I<sub>p</sub></b>	1	in	
Seismic design force	<b>F<sub>p</sub></b>	812.50	lbf	Eq. 13.3-1
Max seismic design force	<b>F<sub>p,max</sub></b>	2600.00	lbf	Eq. 13.3-2
Min seismic design force	<b>F<sub>p,min</sub></b>	487.50	lbf	Eq. 13.3-3
Final seismic design force	<b>F<sub>p</sub></b>	812.50	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 300      **Reviewer:** MT  
**Configuratin No.:** 7      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42250.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	8883.06	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	33366.94	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	1302.12	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42250.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	10454.76	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	31795.24	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	995.94	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1302.12	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.62	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 300      **Reviewer:** MT  
**Configuratin No.:** 7      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1302.12	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.22	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	995.94	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.17	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
 Item: ULT 300  
 Configuratin No.: 7

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	934.38	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	24.00	in	$L = B + 1'$
Capacity reduction factor	f	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = f * 1.4 * M_b$
Demand capacity ratio	DCR	0.16	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1015.63	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4531.25	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 400      **Reviewer:** MT  
**Configuratin No.:** 8      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1600	lbf	per unit cut sheet
Unit width	$B$	28.4	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	31.03	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.84	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.00		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	800.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2560.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	480.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	800.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 400      **Reviewer:** MT  
**Configuratin No.:** 8      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	41600.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	14193.20	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	27406.80	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	883.38	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	41600.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	13857.20	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	27742.80	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	869.00	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	883.38	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.42	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 400      **Reviewer:** MT  
**Configuratin No.:** 8      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	883.38	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.15	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	869.00	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.14	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
 Item: ULT 400  
 Configuratin No.: 8

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	11336.00	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	29.40	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.19	O.K.	$DCR = M_{u2} / \phi M_n$

**Unistrut on front and back sides resisting seismic shear by sideways bending**

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	<input style="width: 100px;" type="text" value="1000.00"/>	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	<input style="width: 100px;" type="text" value="4461.54"/>	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{Sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 500  
**Configuratioin No.:** 9a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1850	lbf	per unit cut sheet
Unit width	$B$	34	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	36.63	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.85		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	786.25	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2516.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	471.75	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	786.25	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 500      **Reviewer:** MT  
**Configuratin No.:** 9a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- | Description | Variable | Value                             | Units | Equation / Reference                         |
|-------------|----------|-----------------------------------|-------|--|
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 5 for x dir |
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 5 for y dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 7 for x dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40885.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	20139.33	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	20745.67	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	566.43	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40885.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	16709.06	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	24175.94	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	757.27	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	757.27	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.36	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 500      **Reviewer:** MT  
**Configuratin No.:** 9a      **Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	566.43	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.09	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	757.27	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 500  
**Configuratin No.:** 9a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1336.63	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	35.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.22	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	982.81	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4384.86	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{Sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT.17  
**Configuratin No.:** 9b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1500	lbf	per unit cut sheet
Unit width	$B$	33.3	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	35.93	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.33	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.10		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	825.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2640.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	495.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.17      **Reviewer:** MT  
**Configuratin No.:** 9b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42845.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	14925.15	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	27919.85	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	777.17	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42845.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	12737.25	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	30107.75	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	937.21	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	937.21	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.45	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.17      **Reviewer:** MT  
**Configuratin No.:** 9b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	777.17	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	937.21	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 17  
**Configuratin No.:** 9b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1373.63	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	34.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.23	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1031.25	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4600.96	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
 Item: ULT 600  
 Configuratin No.: 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	2100	lbf	per unit cut sheet
Unit width	$B$	39.6	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	42.23	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{cg}$	52.00	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.96	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.75		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	787.50	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2520.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	472.50	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	787.50	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)

**Project No.:** 0932

**Item:** ULT 600

**Configuratin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0

**Originator:** JY

**Reviewer:** MT

**Date:** 4/10/2015

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### Seismic Restraints of Freezers

**Notes:**

- Seismic force is determined per Section 13.3 of ASCE 7.
- Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
- In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>
  - (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
  - (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>
  - (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

### 4. Check of Strap

#### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	40950.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	27015.19	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	13934.81	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	330.01	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

#### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	40950.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	19486.69	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	21463.31	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	672.30	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

#### 4.3 Check of Strap

Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	672.30	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.32	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 600      **Reviewer:** MT  
**Configuratin No.:** 10a      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	330.01	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.05	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	672.30	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.11	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
**Configuratin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1559.25	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	40.60	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.26	O.K.	$DCR = M_{u2} / \phi M_n$

### Unistrut on front and back sides resisting seismic shear by sideways bending

### 6. Calculation of Post Installed Anchor Force

#### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$  1784.72 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor	$V_{u2}$	<span style="border: 1px solid black; padding: 2px;">984.38</span> lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	<span style="border: 1px solid black; padding: 2px;">4391.83</span> lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


#### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$  2596.34 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{sb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$  492.19 lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 23  
**Configuratin No.:** 10b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	1650	lbf	per unit cut sheet
Unit width	$B$	40.7	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	43.33	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.07	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.00		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	825.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2640.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	495.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 23      **Reviewer:** MT  
**Configuratin No.:** 10b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- | Description | Variable | Value                             | Units | Equation / Reference                         |
|-------------|----------|-----------------------------------|-------|--|
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 5 for x dir |
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 5 for y dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 7 for x dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42845.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	20319.34	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	22525.66	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	519.92	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42845.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	14423.06	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	28421.94	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	884.73	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	884.73	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.42	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 23      **Reviewer:** MT  
**Configuratin No.:** 10b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	519.92	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.09	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	884.73	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.15	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)

**Project No.:** 0932

**Item:** ULT 23

**Configuratin No.:** 10b

**Cal. No.:** 2015-0932-DC-001, r0

**Originator:** JY

**Reviewer:** MT

**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1678.88	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	41.70	in	
Capacity reduction factor	f	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	
Demand capacity ratio	DCR	0.28	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1031.25	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4600.96	lbf	


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 700  
**Configuratin No.:** 11a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	2500	lbf	per unit cut sheet
Unit width	$B$	45.3	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	47.93	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.53	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.65		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	812.50	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2600.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	487.50	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	812.50	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 700      **Reviewer:** MT  
**Configuratin No.:** 11a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42250.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	37407.56	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	4842.44	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	101.04	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42250.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	23817.06	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	18432.94	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	577.38	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	577.38	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.27	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 700      **Reviewer:** MT  
**Configuratin No.:** 11a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	101.04	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.02	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	577.38	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.10	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 700  
**Configuratin No.:** 11a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1840.31	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	46.30	in	$L = B + 1'$
Capacity reduction factor	f	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = f * 1.4 * M_b$
Demand capacity ratio	DCR	0.31	O.K.	$DCR = M_{u2} / \phi M_n$

**Unistrut on front and back sides resisting seismic shear by sideways bending**

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	<input style="width: 100px;" type="text" value="1015.63"/> lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	<input style="width: 100px;" type="text" value="4531.25"/> lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 28  
**Configuratin No.:** 11b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1800	lbf	per unit cut sheet
Unit width	$B$	46.7	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	49.33	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.67	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.90		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1		
Seismic design force	$F_p$	810.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2592.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	486.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 28      **Reviewer:** MT  
**Configuration No.:** 11b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42066.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	25910.28	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	16155.72	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	327.54	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42066.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	16183.80	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	25882.20	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	805.67	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	805.67	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.38	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 28      **Reviewer:** MT  
**Configuratin No.:** 11b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	327.54	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.05	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	805.67	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
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**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1891.35	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	47.70	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.32	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  1606.79 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1012.50	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4517.31	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$  2769.84 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  506.25 lbf  $V_{u4} = \Omega * F_p / 4$

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## **Section 3. Seismic Restraint Design of Freezers for $I_p = 1.5$ & $z/h=0$**

## Summary of Anchor Force

Configuration	Unit Name	Demand $S_{ps}$	Case 1		Case 2		Case 3	Case 4	Strap Pretension Force <b>lbf</b>
			$T_{u1}$ (lbf)	$V_{u2}$ (lbf)	$T_{u2,prying}$	$T_{u3}$ (lbf)	$V_{u4}$ (lbf)		
1	LRF 12	2.50	2761.31	773.44	3450.72	3226.84	386.72	1239.82	
2	LRF 23	2.30	3426.38	1035.00	4617.69	3752.85	517.50	1420.32	
3	LRF 30	1.80	2700.55	1012.50	4517.31	3596.40	506.25	1314.58	
4a	LRF 45	1.65	1570.55	1020.94	4554.95	4110.54	510.47	1503.85	
4b	LRF 50	1.45	1513.23	1019.53	4548.68	3537.67	509.77	1240.00	
5	LRF 75	1.15	815.78	1035.00	4617.69	3477.33	517.50	1144.80	
6	ULT 13	1.35	2617.53	1025.16	4573.77	3778.74	512.58	1314.41	
7	ULT 300	1.40	3824.27	1023.75	4567.50	3022.64	511.88	1331.31	
8	ULT 400	1.15	3031.59	1035.00	4617.69	2956.20	517.50	949.91	
9a	ULT 500	0.95	2272.39	988.59	4410.65	2711.43	494.30	779.15	
9b	ULT 17	1.20	2524.11	1012.50	4517.31	2888.79	506.25	924.62	
10a	ULT 600	0.85	1850.27	1004.06	4479.66	2676.76	502.03	714.24	
10b	ULT 23	1.10	1991.98	1020.94	4554.95	2864.76	510.47	884.22	
11a	ULT 700	0.70	1365.74	984.38	4391.83	2470.39	492.19	546.35	
11b	ULT 28	1.00	1621.38	1012.50	4517.31	2783.84	506.25	819.67	
<b>Max</b>			<b>3824.27</b>	<b>1035.00</b>	<b>4617.69</b>	<b>4110.54</b>	<b>517.50</b>	<b>1503.85</b>	

$z/h =$	0
$I_p =$	1.5

Note:

- Case 1 Tension load to middle anchor group due to side to side seismic loading
- Case 2 Shear load & tension load by prying action to end anchor due to side to side seismic loading
- Case 3 Tension load to middle anchor group due to front to back seismic loading
- Case 4 Shear loading to end anchor due to front and back seismic loading



**Tobolski Watkins Engineering, Inc.**

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**Project No.:** 0932  
 Item: LRF 12  
**Configuratin No.:** 1

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

<b>W<sub>p</sub></b>	Unit weight	550	lbf	per unit cut sheet
<b>B</b>	Unit width	24	in	per unit cut sheet
<b>D</b>	Unit depth	26.2	in	per unit cut sheet
<b>H</b>	Unit height	73.62	in	per unit cut sheet
<b>D<sub>c</sub></b>	Caster out to out distance	21.3	in	per unit cut sheet
<b>S<sub>ss</sub></b>	Unistrut spacing in side to side direction	26.63	in	$S_{ss} = B + 1" + 1.5/8"$
<b>S<sub>fb</sub></b>	Unistrut spacing in front to back direction	22.93	in	$S_{fb} = D_c + 1.5/8"$
<b>H<sub>eg</sub></b>	CG location	49.08	in	$H_{eg} = H * 2/3$
<b>e<sub>ss</sub></b>	CG eccentricity in side to side direction	2.40	in	$e_{ss} = 0.1B$
<b>e<sub>fb</sub></b>	CG eccentricity in front to back direction	2.62	in	$e_{fb} = 0.1D$
<b>h<sub>unistrut</sub></b>	Height of unistrut plus angle short leg	3.63	in	per unistrut catalogue
<b>d<sub>unistrut</sub></b>	Depth of unistrut	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

<b>S<sub>ps</sub></b>	Short period spectral response acceleration	2.50		normalized height
<b>h</b>	Average roof height of structure	1		normalized height
<b>z</b>	Height in structure of component attachment	0		
<b>a<sub>p</sub></b>	Component response amplification factor	2.5		
<b>R<sub>p</sub></b>	Component response modification factor	6		
<b>Ω</b>	Overstrength factor	2.5		
<b>I<sub>p</sub></b>	Component important factor	1.5		
<b>F<sub>p</sub></b>	Seismic design force	343.75	lbf	Eq. 13.3-1
<b>F<sub>p,max</sub></b>	Max seismic design force	3300.00	lbf	Eq. 13.3-2
<b>F<sub>p,min</sub></b>	Min seismic design force	618.75	lbf	Eq. 13.3-3
<b>F<sub>p</sub></b>	Final seismic design force	618.75	lbf	$\min(F_{p,max}, \max(F_{p,min}))$



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**Item:** LRF 12

**Configuratin No.:** 1

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**Originator:** JY

**Reviewer:** MT

**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- | Description                                    | Variable | Value | Units | Equation / Reference                         |
|--|----------|-------|-------|--|
| (1.2 + 0.2S <sub>DS</sub> )D + ΩE <sub>x</sub> |          |       |       | Section 12.4.3.2 basic load comb 5 for x dir |
| (1.2 + 0.2S <sub>DS</sub> )D + ΩE <sub>y</sub> |          |       |       | Section 12.4.3.2 basic load comb 5 for y dir |
| (0.9 - 0.2S <sub>DS</sub> )D + ΩE <sub>x</sub> |          |       |       | Section 12.4.3.2 basic load comb 7 for x dir |
| (0.9 - 0.2S <sub>DS</sub> )D + ΩE <sub>y</sub> |          |       |       | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction


Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	30368.25	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	2400.75	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	27967.50	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	1050.42	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>SS</sub>

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	30368.25	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	1945.35	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	28422.90	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	1239.82	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

##### 4.3 Check of Strap

Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	1239.82	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.59	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

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**Item:** LRF 12      **Reviewer:** MT  
**Configuratin No.:** 1      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1050.42	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.17	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1239.82	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.21	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	742.50	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	25.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.12	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  2761.31 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	773.44	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	3450.72	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$  3226.84 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{Sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  386.72 lbf  $V_{u4} = \Omega * F_p / 4$





**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
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**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	800	lbf	per unit cut sheet
Unit width	$B$	28	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	30.63	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{eg}$	52.82	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.80	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	2.30		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	460.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4416.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	828.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	828.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 23      **Reviewer:** MT  
**Configuratin No.:** 2      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>      Section 12.4.3.2 basic load comb 5 for x dir  
 (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>      Section 12.4.3.2 basic load comb 5 for y dir  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>      Section 12.4.3.2 basic load comb 7 for x dir  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>      Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction


Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	43734.96	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	4404.40	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	39330.56	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	1284.26	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	43734.96	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	3788.40	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	39946.56	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	1420.32	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

##### 4.3 Check of Strap

Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	1420.32	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.68	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 23  
**Configuratin No.:** 2

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1284.26	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.21	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1420.32	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.24	O.K.	DCR = $M_{u1} / \phi M_n$



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Tobolski Watkins Engineering, Inc.

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 23  
**Configuratin No.:** 2

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1159.20	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	29.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.96		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5791.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.20	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1035.00	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4617.69	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 30  
**Configuratin No.:** 3

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1000	lbf	per unit cut sheet
Unit width	$B$	34	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	36.63	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{eg}$	52.82	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.80		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	450.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4320.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	810.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 30      **Reviewer:** MT  
**Configuratin No.:** 3      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- | Description | Variable | Value                             | Units | Equation / Reference                         |
|-------------|----------|-----------------------------------|-------|--|
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 5 for x dir |
|             |          | $(1.2 + 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 5 for y dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_x$ |       | Section 12.4.3.2 basic load comb 7 for x dir |
|             |          | $(0.9 - 0.2S_{DS})D + \Omega E_y$ |       | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42784.20	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	8052.75	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	34731.45	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	948.30	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42784.20	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	5811.75	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	36972.45	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1314.58	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Description	Variable	Value	Units	Equation / Reference
Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1314.58	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.63	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 30      **Reviewer:** MT  
**Configuratin No.:** 3      **Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	948.30	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1314.58	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.22	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 30  
**Configuratin No.:** 3

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1377.00	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	35.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.94		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5671.30	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.24	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1012.50	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4517.31	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 45  
**Configuratin No.:** 4a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	<b>W<sub>p</sub></b>	1100	lbf	per unit cut sheet
Unit width	<b>B</b>	56.5	in	per unit cut sheet
Unit depth	<b>D</b>	31.5	in	per unit cut sheet
Unit height	<b>H</b>	79.23	in	per unit cut sheet
Caster out to out distance	<b>D<sub>c</sub></b>	23.2	in	per unit cut sheet
Unistrut spacing in side to side direction	<b>S<sub>ss</sub></b>	59.13	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	<b>S<sub>fb</sub></b>	24.83	in	$S_{fb} = D_c + 1.5/8"$
CG location	<b>H<sub>cg</sub></b>	52.82	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	<b>e<sub>ss</sub></b>	5.65	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	<b>e<sub>fb</sub></b>	3.15	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	<b>h<sub>unistrut</sub></b>	3.63	in	per unistrut catalogue
Depth of unistrut	<b>d<sub>unistrut</sub></b>	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	<b>S<sub>ps</sub></b>	1.65		
Average roof height of structure	<b>h</b>	1		normalized height
Height in structure of component attachment	<b>z</b>	0		normalized height
Component response amplification factor	<b>a<sub>p</sub></b>	2.5		
Component response modification factor	<b>R<sub>p</sub></b>	6		
Overstrength factor	<b>Ω</b>	2.5		
Component important factor	<b>I<sub>p</sub></b>	1.5		
Seismic design force	<b>F<sub>p</sub></b>	453.75	lbf	Eq. 13.3-1
Max seismic design force	<b>F<sub>p,max</sub></b>	4356.00	lbf	Eq. 13.3-2
Min seismic design force	<b>F<sub>p,min</sub></b>	816.75	lbf	Eq. 13.3-3
Final seismic design force	<b>F<sub>p</sub></b>	816.75	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 45      **Reviewer:** MT  
**Configuratin No.:** 4a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43140.74	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	14993.14	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	28147.60	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	476.07	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43140.74	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	5807.59	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	37333.15	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1503.85	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1503.85	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.72	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 45      **Reviewer:** MT  
**Configuratin No.:** 4a      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	476.07	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.08	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1503.85	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.25	O.K.	DCR = $M_{u1} / \phi M_n$



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**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 45  
**Configuratin No.:** 4a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	2307.32	lb-ft-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	57.50	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.39	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$  1570.55 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor	$V_{u2}$	<span style="border: 1px solid black; padding: 2px;">1020.94</span> lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	<span style="border: 1px solid black; padding: 2px;">4554.95</span> lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


##### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$  4110.54 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$  510.47 lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1250	lbf	per unit cut sheet
Unit width	$B$	56.5	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	59.13	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	5.65	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.45		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	453.13	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4350.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	815.63	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	815.63	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 50      **Reviewer:** MT  
**Configuratin No.:** 4b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43081.31	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	18233.28	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	24848.03	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	420.26	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43081.31	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	8206.41	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	34874.91	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1240.00	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2) / 2$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1240.00	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.59	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 50      **Reviewer:** MT  
**Configuratin No.:** 4b      **Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	420.26	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.07	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1240.00	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.21	O.K.	DCR = $M_{u1} / \phi M_n$



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Tobolski Watkins Engineering, Inc.

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	2304.14	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	57.50	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.39	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1019.53	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4548.68	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**


**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 75  
**Configuratin No.:** 5

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1600	lbf	per unit cut sheet
Unit width	$B$	85	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	87.63	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.82	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	8.50	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.15		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	460.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4416.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	828.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	828.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 75      **Reviewer:** MT  
**Configuratin No.:** 5      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43734.96	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	37855.00	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	5879.96	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	67.10	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43734.96	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	11537.40	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	32197.56	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1144.80	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1144.80	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.55	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 75      **Reviewer:** MT  
**Configuratin No.:** 5      **Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	67.10	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.01	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1144.80	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.19	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 75  
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**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	3519.00	lb-ft-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	86.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.59	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$  815.78 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor	$V_{u2}$	<span style="border: 1px solid black; padding: 2px;">1035.00</span> lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	<span style="border: 1px solid black; padding: 2px;">4617.69</span> lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


##### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$  3477.33 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$  517.50 lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.13      **Reviewer:** MT  
**Configuratin No.:** 6      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1350	lbf	per unit cut sheet
Unit width	$B$	33.3	in	per unit cut sheet
Unit depth	$D$	29.5	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	24.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	35.93	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	25.93	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.33	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	2.95	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.35		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	455.63	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4374.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	820.13	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	820.13	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

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**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.13      **Reviewer:** MT  
**Configuratin No.:** 6      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42591.83	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	12444.94	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	30146.88	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	839.16	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

#### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42591.83	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	8515.63	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	34076.19	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	1314.41	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

#### 4.3 Check of Strap

Description	Variable	Value	Units	Equation / Reference
Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	1314.41	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.63	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



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**Item:** ULT.13      **Reviewer:** MT  
**Configuratin No.:** 6      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	839.16	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.14	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1314.41	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.22	O.K.	DCR = $M_{u1} / \phi M_n$



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**Project No.:** 0932  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1365.51	lb-ft-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	34.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.23	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$  2617.53 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u2}$  1025.16 lbf  $V_{u2} = \Omega * F_p / 2$   
 Ultimate prying tension force on anchor due to shear  $T_{u2,prying}$  4573.77 lbf  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

##### 6.2 Front to Back Direction


#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$  3778.74 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$  512.58 lbf  $V_{u4} = \Omega * F_p / 4$





**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 300  
**Configuratin No.:** 7

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1300	lbf	per unit cut sheet
Unit width	$B$	23	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	25.63	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.30	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.40		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	455.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4368.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	819.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	819.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



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**Project Name:** Freezers (Thermo Fisher Scientific)  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- | Description                                    | Variable | Value | Units | Equation / Reference                         |
|--|----------|-------|-------|--|
| (1.2 + 0.2S <sub>DS</sub> )D + ΩE <sub>x</sub> |          |       |       | Section 12.4.3.2 basic load comb 5 for x dir |
| (1.2 + 0.2S <sub>DS</sub> )D + ΩE <sub>y</sub> |          |       |       | Section 12.4.3.2 basic load comb 5 for y dir |
| (0.9 - 0.2S <sub>DS</sub> )D + ΩE <sub>x</sub> |          |       |       | Section 12.4.3.2 basic load comb 7 for x dir |
| (0.9 - 0.2S <sub>DS</sub> )D + ΩE <sub>y</sub> |          |       |       | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42588.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	8473.08	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	34114.93	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	1331.31	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42588.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	9972.24	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	32615.77	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	1021.64	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)

##### 4.3 Check of Strap

Description	Variable	Value	Units	Equation / Reference
Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	1331.31	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.63	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
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**Item:** ULT 300      **Reviewer:** MT  
**Configuratin No.:** 7      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	1331.31	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.22	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1021.64	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.17	O.K.	DCR = $M_{u1} / \phi M_n$



**Project Name:** Freezers (Thermo Fisher Scientific) **Cal. No.:** 2015-0932-DC-001, r0  
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**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	941.85	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	24.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.16	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u2}$   lbf  $V_{u2} = \Omega * F_p / 2$   
 Ultimate prying tension force on anchor due to shear  $T_{u2,prying}$   lbf  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


##### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 400  
**Configuratin No.:** 8

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1600	lbf	per unit cut sheet
Unit width	$B$	28.4	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	31.03	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.84	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.15		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	460.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4416.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	828.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	828.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
 Item: ULT 400  
**Configuratin No.:** 8

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- | Description                                    | Variable | Value | Units | Equation / Reference                         |
|--|----------|-------|-------|--|
| (1.2 + 0.2S <sub>DS</sub> )D + ΩE <sub>x</sub> |          |       |       | Section 12.4.3.2 basic load comb 5 for x dir |
| (1.2 + 0.2S <sub>DS</sub> )D + ΩE <sub>y</sub> |          |       |       | Section 12.4.3.2 basic load comb 5 for y dir |
| (0.9 - 0.2S <sub>DS</sub> )D + ΩE <sub>x</sub> |          |       |       | Section 12.4.3.2 basic load comb 7 for x dir |
| (0.9 - 0.2S <sub>DS</sub> )D + ΩE <sub>y</sub> |          |       |       | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	43056.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	13584.92	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	29471.08	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	949.91	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	43056.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	13263.32	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	29792.68	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	933.21	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

##### 4.3 Check of Strap

Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	949.91	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.45	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 400      **Reviewer:** MT  
**Configuratin No.:** 8      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	949.91	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	933.21	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.15	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1175.76	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	29.40	in	$L = B + 1'$
Capacity reduction factor	f	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = f * 1.4 * M_b$
Demand capacity ratio	DCR	0.20	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$  3031.59 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u2}$  1035.00 lbf  $V_{u2} = \Omega * F_p / 2$   
 Ultimate prying tension force on anchor due to shear  $T_{u2,prying}$  4617.69 lbf  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

##### 6.2 Front to Back Direction


#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$  2956.20 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$  517.50 lbf  $V_{u4} = \Omega * F_p / 4$





**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 500  
**Configuratin No.:** 9a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers


- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

### 1. Unit Basic Information

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1850	lbf	per unit cut sheet
Unit width	$B$	34	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	36.63	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{cg}$	52.00	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{ps}$	0.95		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		normalized height
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	439.38	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4218.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	790.88	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	790.88	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 500      **Reviewer:** MT  
**Configuratin No.:** 9a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	41125.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	19587.57	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	21537.93	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	588.07	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	41125.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	16251.28	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	24874.22	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	779.15	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	779.15	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.37	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 500      **Reviewer:** MT  
**Configuratin No.:** 9a      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	588.07	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.10	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	779.15	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 500  
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**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1344.49	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	35.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.23	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  2272.39 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	988.59	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4410.65	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$  2711.43 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  494.30 lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.17      **Reviewer:** MT  
**Configuratin No.:** 9b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1500	lbf	per unit cut sheet
Unit width	$B$	33.3	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	35.93	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.33	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.20		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	450.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4320.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	810.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.17      **Reviewer:** MT  
**Configuratin No.:** 9b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42066.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	14486.18	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	27579.83	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	767.71	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42066.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	12362.63	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	29703.38	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	924.62	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	924.62	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.44	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.17      **Reviewer:** MT  
**Configuratin No.:** 9b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	767.71	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	924.62	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.15	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 17  
**Configuratin No.:** 9b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1348.65	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	34.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.23	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor	$V_{u2}$	<input style="width: 100px;" type="text" value="1012.50"/>
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	<input style="width: 100px;" type="text" value="4517.31"/>

$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

##### 6.2 Front to Back Direction


#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$





**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
**Configuratin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	2100	lbf	per unit cut sheet
Unit width	$B$	39.6	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	42.23	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.96	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.85		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	446.25	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4284.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	803.25	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	803.25	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 600      **Reviewer:** MT  
**Configuratin No.:** 10a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	41769.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	26294.78	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	15474.22	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	366.47	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>SS</sub>

#### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	41769.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	18967.04	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	22801.96	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	714.24	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

#### 4.3 Check of Strap

Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	714.24	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.34	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
**Configuratin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	366.47	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.06	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	714.24	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.12	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)

**Project No.:** 0932

**Item:** ULT 600

**Configuratin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0

**Originator:** JY

**Reviewer:** MT

**Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1590.44	lb-ft-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	40.60	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.27	O.K.	$DCR = M_{u2} / \phi M_n$

### Unistrut on front and back sides resisting seismic shear by sideways bending

### 6. Calculation of Post Installed Anchor Force

#### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$  1850.27 lb  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor	$V_{u2}$	<span style="border: 1px solid black; padding: 2px;">1004.06</span> <span style="margin-left: 20px;">lb</span>
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	<span style="border: 1px solid black; padding: 2px;">4479.66</span> <span style="margin-left: 20px;">lb</span>

$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


#### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$  2676.76 lb  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$  502.03 lb  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 23      **Reviewer:** MT  
**Configuratin No.:** 10b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1650	lbf	per unit cut sheet
Unit width	$B$	40.7	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	43.33	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.07	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.10		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	453.75	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4356.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	816.75	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	816.75	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 23      **Reviewer:** MT  
**Configuratin No.:** 10b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42416.55	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	19738.79	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	22677.77	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	523.43	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42416.55	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	14010.98	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	28405.58	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	884.22	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	884.22	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.42	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 23      **Reviewer:** MT  
**Configuratin No.:** 10b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	523.43	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.09	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	884.22	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.15	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)

**Project No.:** 0932

**Item:** ULT 23

**Configuratin No.:** 10b

**Cal. No.:** 2015-0932-DC-001, r0

**Originator:** JY

**Reviewer:** MT

**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1662.09	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	41.70	in	$L = B + 1'$
Capacity reduction factor	f	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = f * 1.4 * M_b$
Demand capacity ratio	DCR	0.28	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1020.94	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4554.95	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**


**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 700  
**Configuratin No.:** 11a

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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	2500	lbf	per unit cut sheet
Unit width	$B$	45.3	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	47.93	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.53	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.70		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	437.50	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4200.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	787.50	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	787.50	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40950.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	36921.75	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	4028.25	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	84.05	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40950.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	23507.75	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	17442.25	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	546.35	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	546.35	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.26	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 700      **Reviewer:** MT  
**Configuratin No.:** 11a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	84.05	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.01	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	546.35	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.09	O.K.	DCR = $M_{u1} / \phi M_n$



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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1783.69	lb-ft-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	46.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.30	O.K.	$DCR = M_{u2} / \phi M_n$

### Unistrut on front and back sides resisting seismic shear by sideways bending

### 6. Calculation of Post Installed Anchor Force

#### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$  1365.74 lb  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor	$V_{u2}$	<span style="border: 1px solid black; padding: 2px;">984.38</span>	<span style="margin-left: 20px;">lb</span>	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	<span style="border: 1px solid black; padding: 2px;">4391.83</span>	<span style="margin-left: 20px;">lb</span>	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


#### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$  2470.39 lb  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$  492.19 lb  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1800	lbf	per unit cut sheet
Unit width	$B$	46.7	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	49.33	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.67	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.00		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	0		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	450.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	4320.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	810.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- | Description                                    | Variable | Value | Units | Equation / Reference                         |
|--|----------|-------|-------|--|
| (1.2 + 0.2S <sub>DS</sub> )D + ΩE <sub>x</sub> |          |       |       | Section 12.4.3.2 basic load comb 5 for x dir |
| (1.2 + 0.2S <sub>DS</sub> )D + ΩE <sub>y</sub> |          |       |       | Section 12.4.3.2 basic load comb 5 for y dir |
| (0.9 - 0.2S <sub>DS</sub> )D + ΩE <sub>x</sub> |          |       |       | Section 12.4.3.2 basic load comb 7 for x dir |
| (0.9 - 0.2S <sub>DS</sub> )D + ΩE <sub>y</sub> |          |       |       | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42066.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	25190.55	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	16875.45	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	342.13	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42066.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	15734.25	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	26331.75	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	819.67	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

##### 4.3 Check of Strap

Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	819.67	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.39	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



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### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	342.13	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.06	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	819.67	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.14	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)

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**Reviewer:** MT

**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1891.35	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	47.70	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.32	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  1621.38 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1012.50	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4517.31	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$  2783.84 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  506.25 lbf  $V_{u4} = \Omega * F_p / 4$



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## **Section 4. Seismic Restraint Design of Freezers for $I_p = 1.5$ & $z/h=1$**

## Summary of Anchor Force

Configuration	Unit Name	Demand $S_{ps}$	Case 1		Case 2		Case 3	Case 4	Strap Pretension Force
			$T_{u1}$ (lbf)	$V_{u2}$ (lbf)	$T_{u2,prying}$	$T_{u3}$ (lbf)			
1	LRF 12	2.00	3689.26	1031.25	4600.96	4309.52	515.63	1660.17	
2	LRF 23	1.35	3286.67	1012.50	4517.31	3610.18	506.25	1328.35	
3	LRF 30	1.10	2697.63	1031.25	4600.96	3613.25	515.63	1289.17	
4a	LRF 45	1.00	1531.14	1031.25	4600.96	4101.07	515.63	1468.05	
4b	LRF 50	0.85	1410.69	996.09	4444.11	3392.23	498.05	1147.38	
5	LRF 75	0.65	678.96	975.00	4350.00	3190.74	487.50	993.43	
6	ULT 13	0.80	2520.45	1012.50	4517.31	3670.69	506.25	1236.78	
7	ULT 300	0.85	3815.07	1035.94	4621.88	3006.92	517.97	1292.43	
8	ULT 400	0.65	2765.11	975.00	4350.00	2698.73	487.50	804.11	
9a	ULT 500	0.55	2113.63	953.91	4255.89	2541.08	476.95	676.59	
9b	ULT 17	0.70	2381.70	984.38	4391.83	2739.55	492.19	829.94	
10a	ULT 600	0.50	1742.07	984.38	4391.83	2555.65	492.19	631.61	
10b	ULT 23	0.65	1894.60	1005.47	4485.94	2757.02	502.73	806.49	
11a	ULT 700	0.40	1203.20	937.50	4182.69	2259.56	468.75	427.14	
11b	ULT 28	0.60	1563.01	1012.50	4517.31	2727.86	506.25	763.69	
<b>Max</b>			<b>3815.07</b>	<b>1035.94</b>	<b>4621.88</b>	<b>4309.52</b>	<b>517.97</b>	<b>1660.17</b>	

$z/h =$	1
$I_p =$	1.5

Note:

- Case 1 Tension load to middle anchor group due to side to side seismic loading
- Case 2 Shear load & tension load by prying action to end anchor due to side to side seismic loading
- Case 3 Tension load to middle anchor group due to front to back seismic loading
- Case 4 Shear loading to end anchor due to front and back seismic loading



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 12      **Reviewer:** MT  
**Configuratin No.:** 1      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	550	lbf	per unit cut sheet
Unit width	$B$	24	in	per unit cut sheet
Unit depth	$D$	26.2	in	per unit cut sheet
Unit height	$H$	73.62	in	per unit cut sheet
Caster out to out distance	$D_c$	21.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	26.63	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	22.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{cg}$	49.08	in	$H_{cg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	2.62	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	2.00		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	825.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2640.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	495.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



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### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction


Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40491.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	3000.94	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	37490.06	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	1408.08	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40491.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	2431.69	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	38059.31	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1660.17	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1660.17	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.79	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



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**Item:** LRF 12      **Reviewer:** MT  
**Configuratin No.:** 1      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	1408.08	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.23	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1660.17	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.28	O.K.	DCR = $M_{u1} / \phi M_n$



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**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	990.00	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	25.00	in	$L = B + 1'$
Capacity reduction factor	f	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = f * 1.4 * M_b$
Demand capacity ratio	DCR	0.17	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1031.25	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4600.96	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	800	lbf	per unit cut sheet
Unit width	$B$	28	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	30.63	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{eg}$	52.82	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.80	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.35		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	810.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2592.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	486.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



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**Item:** LRF 23      **Reviewer:** MT  
**Configuratin No.:** 2      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42784.20	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	6306.30	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	36477.90	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	1191.12	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42784.20	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	5424.30	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	37359.90	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1328.35	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1328.35	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.63	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Project Name:** Freezers (Thermo Fisher Scientific)  
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**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	1191.12	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.20	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1328.35	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.22	O.K.	DCR = $M_{u1} / \phi M_n$



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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1134.00	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	29.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.96		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5791.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.20	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  3286.67 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1012.50	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4517.31	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$  3610.18 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  506.25 lbf  $V_{u4} = \Omega * F_p / 4$



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**Item:** LRF 30      **Reviewer:** MT  
**Configuratin No.:** 3      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1000	lbf	per unit cut sheet
Unit width	$B$	34	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	36.63	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	1.10		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	825.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2640.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	495.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 30      **Reviewer:** MT  
**Configuratin No.:** 3      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	10140.50	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	33436.00	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	912.93	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	7318.50	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	36258.00	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1289.17	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1289.17	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.61	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 30      **Reviewer:** MT  
**Configuratin No.:** 3      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	912.93	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.15	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1289.17	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.21	O.K.	DCR = $M_{u1} / \phi M_n$



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Tobolski Watkins Engineering, Inc.

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
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**Configuratin No.:** 3

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1402.50	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	35.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.94		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5671.30	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.25	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1031.25	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4600.96	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 45  
**Configuratin No.:** 4a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 1. Unit Basic Information

Unit weight	$W_p$	1100	lbf	per unit cut sheet
Unit width	$B$	56.5	in	per unit cut sheet
Unit depth	$D$	31.5	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	23.2	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	59.13	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	24.83	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{cg}$	52.82	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	5.65	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.15	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{ps}$	1.00		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	825.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2640.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	495.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	825.00	lbf	$\min(F_{p,max}, \max(F_{p,r}, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 45      **Reviewer:** MT  
**Configuratin No.:** 4a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	18412.63	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	25163.88	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	425.60	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43576.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	7132.13	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	36444.38	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1468.05	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1468.05	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.70	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 45      **Reviewer:** MT  
**Configuratin No.:** 4a      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	425.60	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.07	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1468.05	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.24	O.K.	DCR = $M_{u1} / \phi M_n$



**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 45      **Reviewer:** MT  
**Configuratin No.:** 4a      **Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	2330.63	lb-ft-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	57.50	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.39	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning       $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$       1531.14      lbf

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor       $V_{u2} = \Omega * F_p / 2$       1031.25      lbf  
 Ultimate prying tension force on anchor due to shear       $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$       4600.96      lbf


##### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning       $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$       4101.07      lbf

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor       $V_{u4} = \Omega * F_p / 4$       515.63      lbf



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	1250	lbf	per unit cut sheet
Unit width	$B$	56.5	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	59.13	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.82	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	5.65	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.85		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	796.88	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2550.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	478.13	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	796.88	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42090.94	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	21820.16	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	20270.78	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	342.85	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	42090.94	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	9820.78	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	32270.16	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	1147.38	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1147.38	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.55	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 50      **Reviewer:** MT  
**Configuratin No.:** 4b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	342.85	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.06	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1147.38	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.19	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 50  
**Configuratin No.:** 4b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	2251.17	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	57.50	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.38	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	996.09	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4444.11	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** LRF 75  
**Configuratin No.:** 5

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1600	lbf	per unit cut sheet
Unit width	$B$	85	in	per unit cut sheet
Unit depth	$D$	33	in	per unit cut sheet
Unit height	$H$	79.23	in	per unit cut sheet
Caster out to out distance	$D_c$	26.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	87.63	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	28.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.82	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	8.50	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.30	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.65		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	780.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2496.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	468.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	780.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 75      **Reviewer:** MT  
**Configuratin No.:** 5      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	41199.60	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	43505.00	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	-2305.40	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	-26.31	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	41199.60	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	13259.40	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	27940.20	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	993.43	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	993.43	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.47	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 75      **Reviewer:** MT  
**Configuratin No.:** 5      **Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	-26.31	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.00	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	993.43	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** LRF 75      **Reviewer:** MT  
**Configuratin No.:** 5      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	3315.00	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	86.00	in	$L = B + 1'$
Capacity reduction factor	f	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = f * 1.4 * M_b$
Demand capacity ratio	DCR	0.56	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning       $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

678.96      lbf

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor       $V_{u2} = \Omega * F_p / 2$

Ultimate prying tension force on anchor due to shear       $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

975.00      lbf

4350.00      lbf

##### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)


Ultimate tension force on anchor group due to overturning       $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

3190.74      lbf

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor       $V_{u4} = \Omega * F_p / 4$

487.50      lbf



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.13      **Reviewer:** MT  
**Configuratin No.:** 6      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1350	lbf	per unit cut sheet
Unit width	$B$	33.3	in	per unit cut sheet
Unit depth	$D$	29.5	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	24.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	35.93	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	25.93	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{cg}$	51.93	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.33	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	2.95	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.80		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	810.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2592.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	486.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.13      **Reviewer:** MT  
**Configuratin No.:** 6      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42066.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	14617.87	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	27448.13	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	764.04	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

#### 4.2 Front to Back Direction

Overturning moment due to seismic force	M <sub>OT</sub>	42066.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	10002.49	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	32063.51	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	1236.78	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

#### 4.3 Check of Strap

Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	1236.78	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.59	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.13      **Reviewer:** MT  
**Configuratin No.:** 6      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	764.04	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	1236.78	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.20	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
 Item: ULT.13  
 Configuratioin No.: 6

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1348.65	lb-ft-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	34.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.23	O.K.	$DCR = M_{u2} / \phi M_n$

**Unistrut on front and back sides resisting seismic shear by sideways bending**

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  2520.45 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	<span style="border: 1px solid black; padding: 2px;">1012.50</span> lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	<span style="border: 1px solid black; padding: 2px;">4517.31</span> lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$  3670.69 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  506.25 lbf  $V_{u4} = \Omega * F_p / 4$



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**Project No.:** 0932  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1300	lbf	per unit cut sheet
Unit width	$B$	23	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	25.63	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.30	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.85		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	828.75	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2652.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	497.25	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	828.75	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 300      **Reviewer:** MT  
**Configuratin No.:** 7      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$       Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$       Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43095.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	9976.36	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	33118.64	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	1292.43	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	43095.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	11741.50	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	31353.50	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	982.10	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	1292.43	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	O.K.	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .





**Project Name:** Freezers (Thermo Fisher Scientific)  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	1292.43	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.21	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	982.10	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
- Seismic force is determined per Section 13.3 of ASCE 7.
  - Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  - In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### Unistrut on front and back sides resisting seismic shear by sideways bending

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	953.06	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	24.00	in	
Capacity reduction factor	f	0.99		L = B+1" per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = f * 1.4 * M_b$
Demand capacity ratio	DCR	0.16	O.K.	DCR = $M_{u2} / \phi M_n$

## 6. Calculation of Post Installed Anchor Force

### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor	$V_{u2}$	1035.94	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4621.88	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
 Item: ULT 400  
 Configuratin No.: 8

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Unit weight	$W_p$	1600	lbf	per unit cut sheet
Unit width	$B$	28.4	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	31.03	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	2.84	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.65		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	780.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2496.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	468.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	780.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 400      **Reviewer:** MT  
**Configuration No.:** 8      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 5 for x dir  
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 5 for y dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 Section 12.4.3.2 basic load comb 7 for x dir  
 $(0.9 - 0.2S_{DS})D + \Omega E_y$   
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40560.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	15612.52	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	24947.48	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	804.11	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40560.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	15242.92	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	25317.08	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	793.02	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	804.11	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.38	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



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**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
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**Item:** ULT 400      **Reviewer:** MT  
**Configuratin No.:** 8      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	804.11	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	793.02	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
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**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1107.60	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	29.40	in	$L = B + 1'$
Capacity reduction factor	f	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = f * 1.4 * M_b$
Demand capacity ratio	DCR	0.19	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	975.00	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4350.00	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 500  
**Configuratin No.:** 9a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1850	lbf	per unit cut sheet
Unit width	$B$	34	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	36.63	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.40	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.55		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	763.13	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2442.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	457.88	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	763.13	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 500      **Reviewer:** MT  
**Configuration No.:** 9a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	39682.50	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	21794.62	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	17887.88	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	488.41	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

#### 4.2 Front to Back Direction

Overturning moment due to seismic force	M <sub>OT</sub>	39682.50	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	18082.41	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	21600.09	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	676.59	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

#### 4.3 Check of Strap

Tested allowable capacity of strap Max strap force Demand capacity ratio	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> ) DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.
	F <sub>max</sub>	676.59	lbf	
	DCR	0.32	O.K.	





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
 Item: ULT 500  
 Configuratin No.: 9a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	488.41	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.08	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	676.59	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.11	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 500  
**Configuratin No.:** 9a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1297.31	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	35.00	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.22	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	953.91	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4255.89	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.17      **Reviewer:** MT  
**Configuratin No.:** 9b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1500	lbf	per unit cut sheet
Unit width	$B$	33.3	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	35.93	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.33	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.70		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	787.50	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2520.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	472.50	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	787.50	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.17      **Reviewer:** MT  
**Configuratin No.:** 9b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40897.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	16681.05	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	24216.45	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	674.08	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40897.50	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	14235.75	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	26661.75	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	829.94	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	829.94	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.40	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT.17      **Reviewer:** MT  
**Configuratin No.:** 9b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	674.08	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.11	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	829.94	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.14	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT.17  
**Configuratin No.:** 9b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1311.19	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	34.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.22	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  2381.70 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	984.38	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4391.83	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$  2739.55 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  492.19 lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
**Configuratin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 1. Unit Basic Information

Unit weight	$W_p$	2100	lbf	per unit cut sheet
Unit width	$B$	39.6	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	42.23	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{cg}$	52.00	in	$H_{cg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	3.96	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

### 2. Seismic Force Calculation

Short period spectral response acceleration	$S_{ps}$	0.50		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	787.50	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2520.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	472.50	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	787.50	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
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**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- |  |                                   |  |
|--|-----------------------------------|--|
|  | $(1.2 + 0.2S_{DS})D + \Omega E_x$ | Section 12.4.3.2 basic load comb 5 for x dir |
|  | $(1.2 + 0.2S_{DS})D + \Omega E_y$ | Section 12.4.3.2 basic load comb 5 for y dir |
|  | $(0.9 - 0.2S_{DS})D + \Omega E_x$ | Section 12.4.3.2 basic load comb 7 for x dir |
|  | $(0.9 - 0.2S_{DS})D + \Omega E_y$ | Section 12.4.3.2 basic load comb 7 for y dir |

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	40950.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	28816.20	lbf-in	$M_{R,grav} = (0.9-0.2*S_{DS})W_p * (S_{ss}/2-e_{ss})$
Resisting moment by strap	$M_{R,strap}$	12133.80	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	287.36	lbf	$F_{ss} = M_{R,strap}/S_{ss}$


##### 4.2 Front to Back Direction

Overturning moment due to seismic force	$M_{OT}$	40950.00	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	20785.80	lbf-in	$M_{R,grav} = (0.9-0.2*S_{DS})W_p * (S_{fb}/2-e_{fb})$
Resisting moment by strap	$M_{R,strap}$	20164.20	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	631.61	lbf	$F_{fb} = M_{R,strap}/(S_{fb}/2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	631.61	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.30	O.K.	$DCR = F_{max}/(1.4T_a)$ , O.K. if DCR < 1.





**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 600      **Reviewer:** MT  
**Configuratin No.:** 10a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	287.36	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.05	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	631.61	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.10	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 600  
**Configuratin No.:** 10a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1559.25	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	40.60	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.26	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	984.38	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4391.83	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{sb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 23  
**Configuratin No.:** 10b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	1650	lbf	per unit cut sheet
Unit width	$B$	40.7	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	43.33	in	$S_{ss} = B+1" +1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c+1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.07	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.65		
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		normalized height
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	804.38	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2574.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	482.63	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	804.38	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 23      **Reviewer:** MT  
**Configuratin No.:** 10b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- $(1.2 + 0.2S_{DS})D + \Omega E_x$   
 $(1.2 + 0.2S_{DS})D + \Omega E_y$   
 $(0.9 - 0.2S_{DS})D + \Omega E_x$   
 $(0.9 - 0.2S_{DS})D + \Omega E_y$
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap

##### 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	41773.88	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	22351.27	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{ss} / 2 - e_{ss})$
Resisting moment by strap	$M_{R,strap}$	19422.60	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for side to side direction	$F_{ss}$	448.30	lbf	$F_{ss} = M_{R,strap} / S_{ss}$

##### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	$M_{OT}$	41773.88	lbf-in	$M_{OT} = F_p * H_{cg}$
Resisting moment by gravity	$M_{R,grav}$	15865.37	lbf-in	$M_{R,grav} = (0.9 - 0.2 * S_{DS}) W_p * (S_{fb} / 2 - e_{fb})$
Resisting moment by strap	$M_{R,strap}$	25908.51	lbf-in	$M_{R,strap} = M_{OT} - M_{R,grav}$
Required strap force for front to back direction	$F_{fb}$	806.49	lbf	$F_{fb} = M_{R,strap} / (S_{fb} / 2)$

##### 4.3 Check of Strap

Tested allowable capacity of strap	$T_a$	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	$F_{max}$	806.49	lbf	$F_{max} = \max(F_{ss}, F_{fb})$
Demand capacity ratio	DCR	0.38	O.K.	$DCR = F_{max} / (1.4T_a)$ , O.K. if $DCR < 1$ .



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 23      **Reviewer:** MT  
**Configurator No.:** 10b      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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#### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

#### 5.1 Side to Side Direction

##### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	448.30	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.07	O.K.	DCR = $M_{u1} / \phi M_n$

##### Unistrut on the other side resisting seismic shear by anchor shear

#### 5.2 Front to Back Direction

##### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	806.49	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$



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Tobolski Watkins Engineering, Inc.

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 23  
**Configuratin No.:** 10b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1636.90	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	41.70	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.27	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  1894.60 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1005.47	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4485.94	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$


**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u3}$  2757.02 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  502.73 lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 700  
**Configuratin No.:** 11a

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
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**1. Unit Basic Information**

Description	Variable	Value	Units	Equation / Reference
Unit weight	$W_p$	2500	lbf	per unit cut sheet
Unit width	$B$	45.3	in	per unit cut sheet
Unit depth	$D$	35.9	in	per unit cut sheet
Unit height	$H$	78	in	per unit cut sheet
Caster out to out distance	$D_c$	30.3	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	47.93	in	$S_{ss} = B+1''+1.5/8''$
Unistrut spacing in front to back direction	$S_{fb}$	31.93	in	$S_{fb} = D_c+1.5/8''$
CG location	$H_{eg}$	52.00	in	$H_{eg} = H*2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.53	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.59	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.40		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component repsonse amplification factor	$a_p$	2.5		
Component repsonse modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	750.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2400.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	450.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	750.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 700      **Reviewer:** MT  
**Configuratin No.:** 11a      **Date:** 4/10/2015

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	39000.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	39836.63	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	-836.62	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	-17.46	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

#### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	39000.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	25363.63	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	13636.38	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	427.14	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

#### 4.3 Check of Strap

Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	427.14	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.20	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.





**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 700      **Reviewer:** MT  
**Configuratin No.:** 11a      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction


Ultimate moment in unistrut due to strap force	$M_{u1}$	-17.46	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.00	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	427.14	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.07	O.K.	DCR = $M_{u1} / \phi M_n$



**Project Name:** Freezers (Thermo Fisher Scientific) **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932 **Originator:** JY  
**Item:** ULT 700 **Reviewer:** MT  
**Configuratin No.:** 11a **Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1698.75	lb-ft	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	L	46.30	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lb-ft	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.28	O.K.	$DCR = M_{u2} / \phi M_n$

#### Unistrut on front and back sides resisting seismic shear by sideways bending

#### 6. Calculation of Post Installed Anchor Force

##### 6.1 Side to Side Direction

#### Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u1}$   lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

#### End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u2}$   lbf  $V_{u2} = \Omega * F_p / 2$   
 Ultimate prying tension force on anchor due to shear  $T_{u2,prying}$   lbf  $T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

##### 6.2 Front to Back Direction

#### Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)

Ultimate tension force on anchor group due to overturning  $T_{u3}$   lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{sb} / 2) / 2$

#### Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)

Ultimate shear force on anchor  $V_{u4}$   lbf  $V_{u4} = \Omega * F_p / 4$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 28  
**Configuratin No.:** 11b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.


Description	Variable	Value	Units	Equation / Reference
-------------	----------	-------	-------	----------------------

**1. Unit Basic Information**

Unit weight	$W_p$	1800	lbf	per unit cut sheet
Unit width	$B$	46.7	in	per unit cut sheet
Unit depth	$D$	35.75	in	per unit cut sheet
Unit height	$H$	77.9	in	per unit cut sheet
Caster out to out distance	$D_c$	30.5	in	per unit cut sheet
Unistrut spacing in side to side direction	$S_{ss}$	49.33	in	$S_{ss} = B + 1" + 1.5/8"$
Unistrut spacing in front to back direction	$S_{fb}$	32.13	in	$S_{fb} = D_c + 1.5/8"$
CG location	$H_{eg}$	51.93	in	$H_{eg} = H * 2/3$
CG eccentricity in side to side direction	$e_{ss}$	4.67	in	$e_{ss} = 0.1B$
CG eccentricity in front to back direction	$e_{fb}$	3.58	in	$e_{fb} = 0.1D$
Height of unistrut plus angle short leg	$h_{unistrut}$	3.63	in	per unistrut catalogue
Depth of unistrut	$d_{unistrut}$	1.63	in	per unistrut catalogue

**2. Seismic Force Calculation**

Short period spectral response acceleration	$S_{ps}$	0.60		normalized height
Average roof height of structure	$h$	1		normalized height
Height in structure of component attachment	$z$	1		
Component response amplification factor	$a_p$	2.5		
Component response modification factor	$R_p$	6		
Overstrength factor	$\Omega$	2.5		
Component important factor	$I_p$	1.5		
Seismic design force	$F_p$	810.00	lbf	Eq. 13.3-1
Max seismic design force	$F_{p,max}$	2592.00	lbf	Eq. 13.3-2
Min seismic design force	$F_{p,min}$	486.00	lbf	Eq. 13.3-3
Final seismic design force	$F_p$	810.00	lbf	$\min(F_{p,max}, \max(F_p, F_{p,min}))$



**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
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**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

**Tobolski Watkins Engineering, Inc.**

### Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

#### 3. Load Combinations

- (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (1.2 + 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>x</sub>  
 (0.9 - 0.2S<sub>DS</sub>)D + ΩE<sub>y</sub>
- Section 12.4.3.2 basic load comb 5 for x dir  
 Section 12.4.3.2 basic load comb 5 for y dir  
 Section 12.4.3.2 basic load comb 7 for x dir  
 Section 12.4.3.2 basic load comb 7 for y dir

#### 4. Check of Strap 4.1 Side to Side Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42066.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	28069.47	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>ss</sub> /2-e <sub>ss</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	13996.53	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for side to side direction	F <sub>ss</sub>	283.76	lbf	F <sub>ss</sub> = M <sub>R,strap</sub> /S <sub>ss</sub>

#### 4.2 Front to Back Direction

Description	Variable	Value	Units	Equation / Reference
Overturning moment due to seismic force	M <sub>OT</sub>	42066.00	lbf-in	M <sub>OT</sub> = F <sub>p</sub> * H <sub>cg</sub>
Resisting moment by gravity	M <sub>R,grav</sub>	17532.45	lbf-in	M <sub>R,grav</sub> = (0.9-0.2*S <sub>DS</sub> )*W <sub>p</sub> * (S <sub>fb</sub> /2-e <sub>fb</sub> )
Resisting moment by strap	M <sub>R,strap</sub>	24533.55	lbf-in	M <sub>R,strap</sub> = M <sub>OT</sub> - M <sub>R,grav</sub>
Required strap force for front to back direction	F <sub>fb</sub>	763.69	lbf	F <sub>fb</sub> = M <sub>R,strap</sub> /(S <sub>fb</sub> /2)/2

#### 4.3 Check of Strap

Tested allowable capacity of strap	T <sub>a</sub>	1500	lbf	per information of QuakHold Part No. 44627-11 with 2" Strap
Max strap force	F <sub>max</sub>	763.69	lbf	F <sub>max</sub> = max(F <sub>ss</sub> , F <sub>fb</sub> )
Demand capacity ratio	DCR	0.36	O.K.	DCR = F <sub>max</sub> /(1.4T <sub>a</sub> ), O.K. if DCR < 1.



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)      **Cal. No.:** 2015-0932-DC-001, r0  
**Project No.:** 0932      **Originator:** JY  
**Item:** ULT 28      **Reviewer:** MT  
**Configuratin No.:** 11b      **Date:** 4/10/2015

## Sesimic Restraints of Freezers

- Notes:**
1. Seismic force is determined per Section 13.3 of ASCE 7.
  2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
  3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
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### 5. Check of Unistrut

Unistrut allowable bending capacity for any dir. bending	$M_a$	5070	lbf-in	for 1 5/8in unistrut P1000 12GA
Modified bending capacity for HS/T Section	$M_a$	4309.5	lbf-in	for 1 5/8in unistrut P1000T 12GA
Max spacing of anchors next to strap	$S_{anchor}$	4	in	
Max distance of end anchors to the end of unistrut	$D_{anchor}$	6.5	in	

### 5.1 Side to Side Direction

#### Unistrut on one side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	283.76	lbf-in	$M_{u1} = F_{ss} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.05	O.K.	DCR = $M_{u1} / \phi M_n$

#### Unistrut on the other side resisting seismic shear by anchor shear

### 5.2 Front to Back Direction

#### Unistrut on each side resisting strap force by bending in vertical direction

Ultimate moment in unistrut due to strap force	$M_{u1}$	763.69	lbf-in	$M_{u1} = F_{fb} * S_{anchor} / 4$
Unbraced length	L	4	in	
Capacity reduction factor	f	1		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	6033.30	lbf-in	$\phi M_n = f * 1.4 * M_a$
Demand capacity ratio	DCR	0.13	O.K.	DCR = $M_{u1} / \phi M_n$



**Tobolski Watkins Engineering, Inc.**

**Project Name:** Freezers (Thermo Fisher Scientific)  
**Project No.:** 0932  
**Item:** ULT 28  
**Configuratin No.:** 11b

**Cal. No.:** 2015-0932-DC-001, r0  
**Originator:** JY  
**Reviewer:** MT  
**Date:** 4/10/2015

## Sesimic Restraints of Freezers

**Notes:**

1. Seismic force is determined per Section 13.3 of ASCE 7.
2. Strap is tied down by eye bolt to unistrut, which is anchored to slab below by (2) post installed anchors of max 4in spacing.
3. In addition, for unistrut on the side, one post installed anchor is provided at max 6.5in from each end.

Description	Variable	Value	Units	Equation / Reference
-------------	----------	-------	-------	----------------------

**Unistrut on front and back sides resisting seismic shear by sideways bending**

Ultimate moment in unistrut due to seismic shear	$M_{u2}$	1891.35	lbf-in	$M_{u2} = F_p/2 * 0.1 * B$
Unbraced length	$L$	47.70	in	$L = B + 1'$
Capacity reduction factor	$\phi$	0.99		per unistrut catalog page 56 for P1000 beam.
Unistrut ultimate bending capacity	$\phi M_n$	5972.97	lbf-in	$\phi M_n = \phi * 1.4 * M_b$
Demand capacity ratio	DCR	0.32	O.K.	$DCR = M_{u2} / \phi M_n$

**6. Calculation of Post Installed Anchor Force**

**6.1 Side to Side Direction**

**Anchor bolts of unistrut on one side resisting tension only (Case 1 for anchor check for an anchor group)**

Ultimate tension force on anchor group due to overturning  $T_{u1}$  1563.01 lbf  $T_{u1} = (\Omega * M_{OT} - M_{R,grav}) / S_{Ss}$

**End anchor bolts of unistrut on the other side resisting shear only with prying action (Case 2 for anchor check for individual anchor)**

Ultimate shear force on anchor	$V_{u2}$	1012.50	lbf	$V_{u2} = \Omega * F_p / 2$
Ultimate prying tension force on anchor due to shear	$T_{u2,prying}$	4517.31	lbf	$T_{u2,prying} = V_{u2} * h_{unistrut} / (d_{unistrut} / 2)$

**6.2 Front to Back Direction**

**Anchor bolts of unistruts on both sides resisting tension and shear (Case 3 for anchor check for an anchor group)**


Ultimate tension force on anchor group due to overturning  $T_{u3}$  2727.86 lbf  $T_{u3} = (\Omega * M_{OT} - M_{R,grav}) / (S_{fb} / 2) / 2$

**Anchor bolts of unistruts on both sides resisting shear (Case 4 for anchor check for individual anchor)**

Ultimate shear force on anchor  $V_{u4}$  506.25 lbf  $V_{u4} = \Omega * F_p / 4$

---

## **Section 5. Check of Middle Anchor**

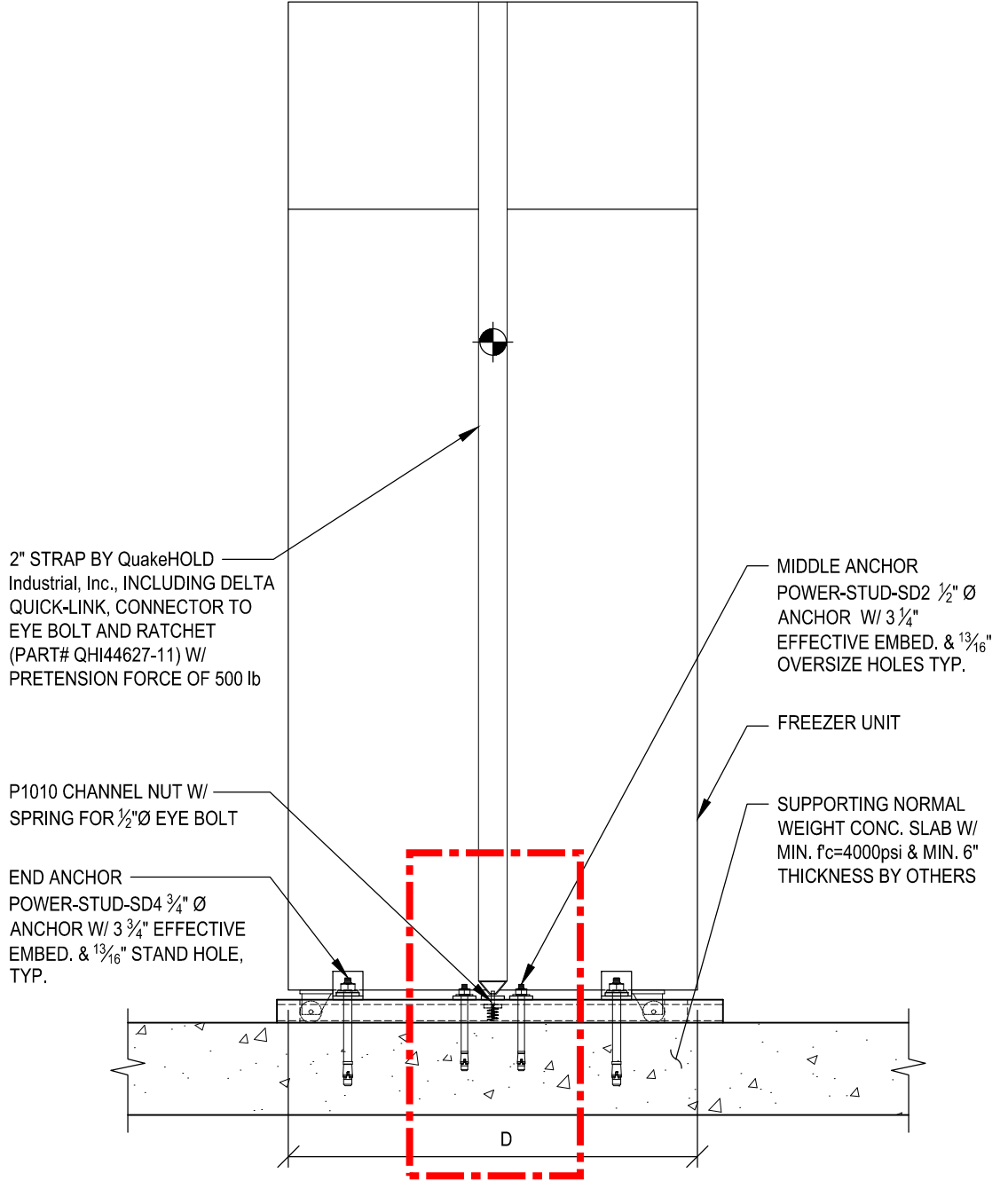
	Company name:		Date:	7/14/2015
	Project:		Page:	1/5
	Version: 2.2.5543.30490	Project number:		

**PROJECT DATA:**

Company name:  
 Project engineer:  
 Project name:  
 Subject/Topic: **Middle Anchor Capacity Check**

**Description:**

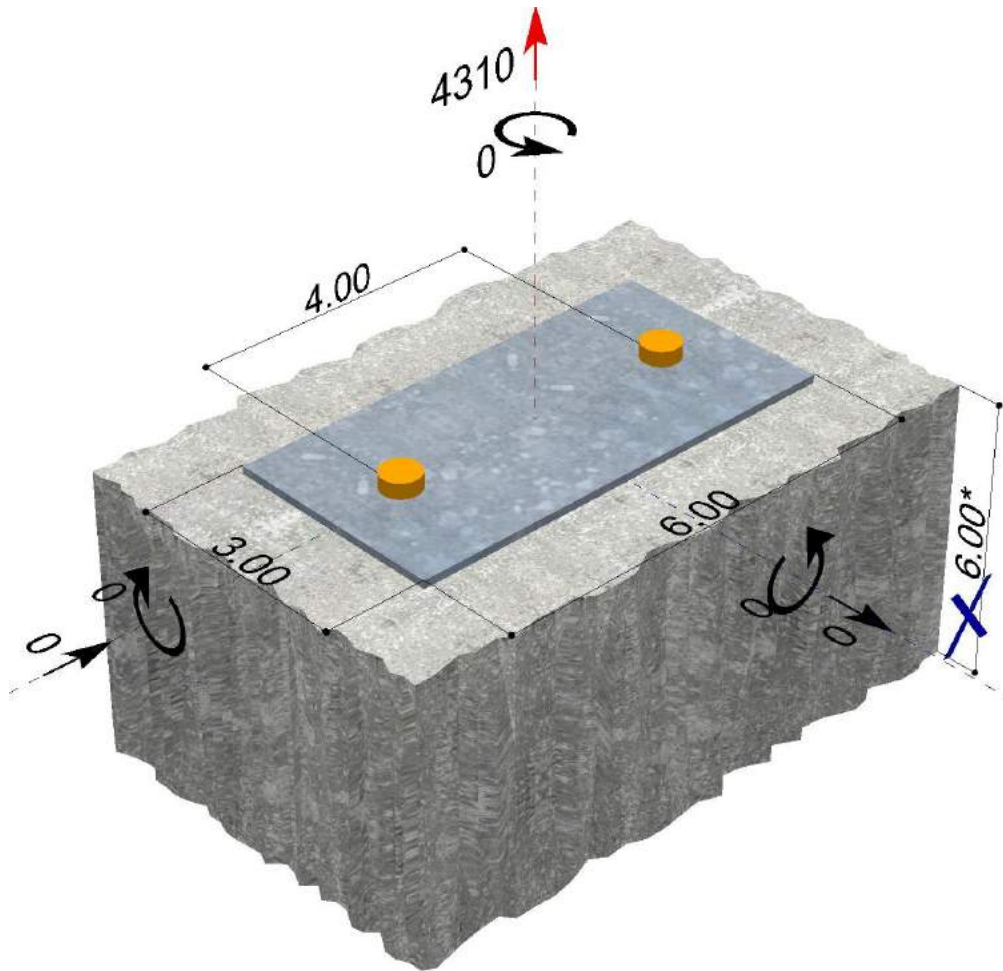
**Notes/Remarks:**



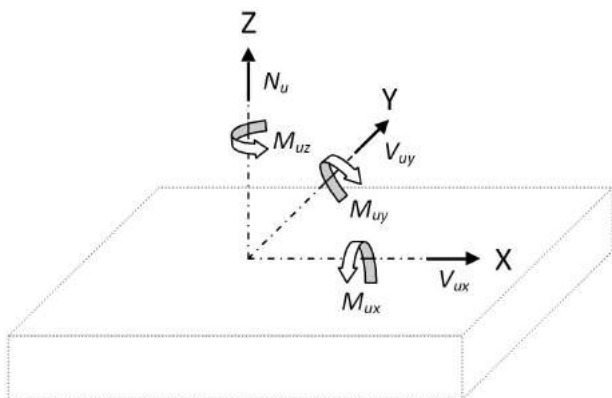
3 SIDE ELEVATION VIEW



**GEOMETRY:**




**LOAD ACTIONS: [lb], [ft-lb]**



Design loads / actions		
$N_u$	4310	lb
$V_{ux}$	0	lb
$V_{uy}$	0	lb
$M_{ux}$	0	ft-lb
$M_{uy}$	0	ft-lb
$M_{uz}$	0	ft-lb

Eccentric profile		
$e_x = 0.00$ inch; $e_y = 0.00$ inch		
Load reversal	100	%
X-direction		
Load reversal	100	%
Y-direction		

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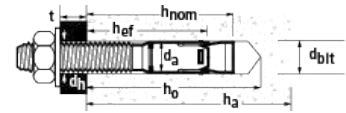
**SUMMARY:**

Selected anchor: Power-Stud+ SD2  
 1/2" Ø; h<sub>nom</sub> 3-3/4" (95mm), Grade 2

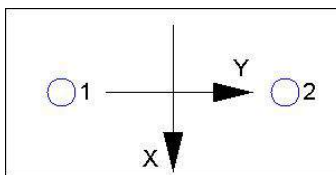
Effective embedment depth: h<sub>ef</sub> = 3.250 inch

Approval: ESR-2502

Issued: 5/1/2014

**Basic principles of Design:**

Design method:	ACI 318-11 (Appendix D)		
Concrete:	Normal weight concrete	cracked concrete	f' <sub>c</sub> = 4000 psi
Load combination:	taken from Section 9.2		
	Factored loads		
	Ω = User enters load		
Anchor Parameters:	c <sub>min</sub> = 2.75 inch	s <sub>min</sub> = 4.00 inch	h <sub>min</sub> = 5.75 inch
	c <sub>ac</sub> = 10.00 inch	s <sub>cr</sub> = 9.75 inch	Anchor Ductility: Yes
Reinforcement:	none edge reinforcement or < #4 bar		
	Tension: Condition B	Shear: Condition B	
Stand-off:	not existent		
Seismic Loads:	Yes		
	Tension load	Yes (D.3.3.4.3(d))	
	Shear load	Yes (D.3.3.5.3(c))	

**Resulting anchor forces / load distribution::**

Anchor No.	Tension load	Shear load
#1	2155 lb	0 lb
#2	2155 lb	0 lb
Maximum	2155 lb	0 lb

Max. concrete compression strain: 0.00 ‰

Max. concrete compression stress: 0 psi

Resulting tension force: 4310 lb

Resulting compression force: 0 lb

Calculations:	Design proof:	Demand	Capacity	Status
	Tension load	4310 lb	4331 lb	1.00 ≤ 1.0
	Shear load	-	-	-
	Interaction	-	-	-
				OK

**Anchor plate:**

Material: f<sub>yk</sub> = 36000 psi

Length x width: 3.00 inch x 6.00 inch

Actual plate thickness: 0.000 inch

Calculated plate thickness: - inch not calculated

**Profile:** none selected

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines and must be checked for plausibility.

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Date: 7/14/2015

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Project number:

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**DESIGN PROOF TENSION LOADING:****Reference****Steel strength:**

$$N_{sa} = 10445 \text{ lb} \quad \text{D.5.1}$$

$$\Phi * N_{sa} = \Phi * N_{sa} \quad \text{D.5.1.2}$$

$$= 0.75 * 10445 \text{ lb} = 7834 \text{ lb}$$

$$N_{ua} = 2155 \text{ lb}$$

$$\text{Design proof: } N_{ua} / (\Phi * N_{sa}) = 2155 \text{ lb} / 7834 \text{ lb} = 0.28 \leq 1.00$$

**Concrete Breakout Strength:**

$$h_{ef} = 3.250 \text{ inch}$$

$$k_c = 17.0$$

$$N_b = k_c * f'_c{}^{0.5} * \lambda_a * h_{ef}{}^{1.5} \quad \text{D.5.2.2}$$

$$= 17.0 * 63.25 * 1.00 * 5.859 = 6299 \text{ lb}$$

$$A_{Nc0} = 95.06 \text{ inch}^2$$

$$A_{Nc} = 134.06 \text{ inch}^2$$

$$\psi_{ec,N,x} = 1.000 \quad \text{D.5.2.4}$$

$$\psi_{ec,N,y} = 1.000 \quad \text{D.5.2.4}$$

$$\psi_{ed,N} = 1.000 \quad \text{D.5.2.5}$$

$$\psi_{c,N} = 1.00 \quad \text{D.5.2.6}$$

$$c_{ac} = 10.00 \text{ inch}$$

$$\psi_{cp,N} = 1.000 \quad \text{D.5.2.7}$$

$$\Phi * N_{cbg} = \Phi_{seis} * \Phi * (A_{Nc} / A_{Nc0}) * \psi_{ec,N,x} * \psi_{ec,N,y} * \psi_{ed,N} * \psi_{c,N} * \psi_{cp,N} * N_b \quad \text{D.5.2.1}$$

$$= 0.75 * 0.65 * (134.06 / 95.06) * 1.000 * 1.000 * 1.000 * 1.00 * 1.000 * 6299 \text{ lb}$$

$$= 4331 \text{ lb}$$

$$N_{ua} = 4310 \text{ lb}$$

$$\text{Design proof: } N_{ua} / (\Phi * N_{cbg}) = 4310 \text{ lb} / 4331 \text{ lb} = 1.00 \leq 1.00$$

**Pullout / Bond strength:**

$$N_{p,eq} = 4375 \text{ lb} \quad \text{D.5.3.2}$$

$$\Phi * N_{pn} = \Phi_{seis} * \Phi * (f'_c / 2500)^{0.5} * N_{p,eq}$$

$$= 0.75 * 0.65 * (4000 / 2500)^{0.5} * 4375 = 2698 \text{ lb}$$


$$N_{ua} = 2155 \text{ lb}$$

$$\text{Design proof: } N_{ua} / (\Phi * N_{pn}) = 2155 \text{ lb} / 2698 \text{ lb} = 0.80 \leq 1.00$$

**Fastening ok!**

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines and must be checked for plausibility.

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Version: 2.2.5543.30490	Project number:			

**WARNINGS / REMARKS:**

Calculations including seismic design requirements in accordance with ACI 318 D.3.3 are required for anchors in structures assigned to seismic design categories C, D, E and F. Under these seismic conditions, the direction of shear may not be predictable. As default and in accordance with ACI 318 D.3.3 the full shear force is assumed also in reverse direction for a safe design. This may influence the direction of the controlling concrete breakout strength.


D.3.3.4.3 (c) Selected:

Anchor or anchor group shall be designed for the max design load combinations that include E, with E increased by  $\Omega_o$ . The anchor design tensile strength shall satisfy the tensile strength requirements of D.3.3.4.4.

Per ACI 318-11, Part D.3.3.4.3(b) the anchor or group of anchors shall be designed for the maximum tension that can be transmitted to the anchor or group of anchors based on the development of a ductile yield mechanism in the attachment in flexure, shear, or bearing, or a combination of those conditions, and considering both material overstrength and strain hardening effects for the attachment. The anchor design tensile strength shall be calculated from D.3.3.4.4. Per ACI 318-11, Part D.3.3.4.3(c) the anchor or group of anchors shall be designed for the maximum tension that can be transmitted to the anchors by a non-yielding attachment. The anchor design tensile strength shall be calculated from D.3.3.4.4. Per ACI 318-11, Part D.3.3.4.3(d) The anchor or group of anchors shall be designed for the maximum tension obtained from design load combinations that include E, with E increased by  $\Omega_o$ . The anchor design tensile strength shall satisfy the tensile strength requirements of D.4.1.1.

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## **Section 6. Check of End Anchors & Angle Brackets**

	Company name:		Date:	7/14/2015
	Project:		Page:	1/6
	Version: 2.2.5543.30490	Project number:		

**PROJECT DATA:**

Company name:  
 Project engineer:  
 Project name:  
 Subject/Topic: **End Anchor Capacity Check**

**Description:**

**Notes/Remarks:**

2" STRAP BY QuakeHOLD Industrial, Inc., INCLUDING DELTA QUICK-LINK, CONNECTOR TO EYE BOLT AND RATCHET (PART# QH144627-11) W/ PRETENSION FORCE OF 500 lb

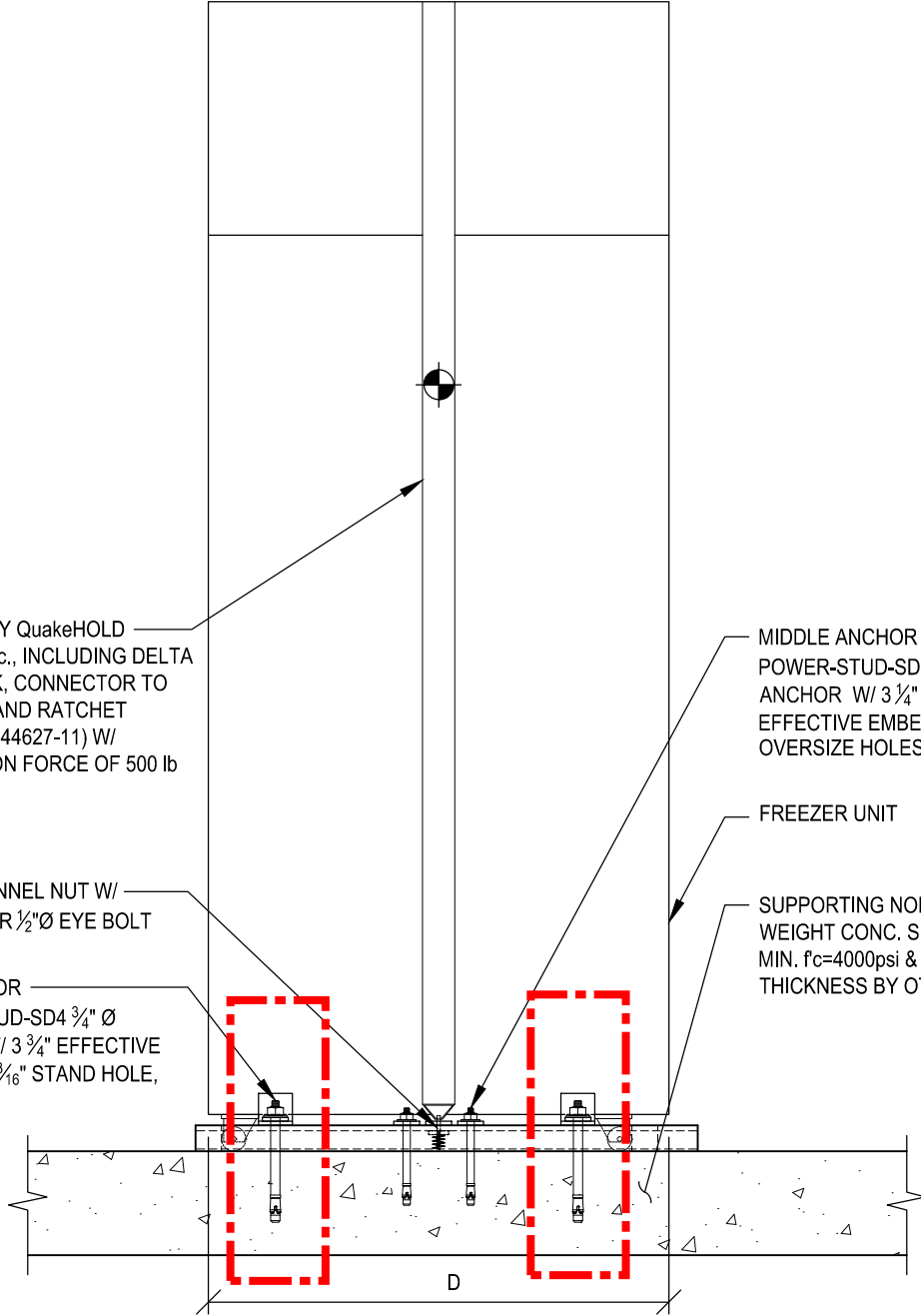
P1010 CHANNEL NUT W/ SPRING FOR 1/2" Ø EYE BOLT

END ANCHOR POWER-STUD-SD4 3/4" Ø ANCHOR W/ 3 3/4" EFFECTIVE EMBED. & 13/16" STAND HOLE, TYP.

MIDDLE ANCHOR POWER-STUD-SD2 1/2" Ø ANCHOR W/ 3 1/4" EFFECTIVE EMBED. & 13/16" OVERSIZE HOLES TYP.

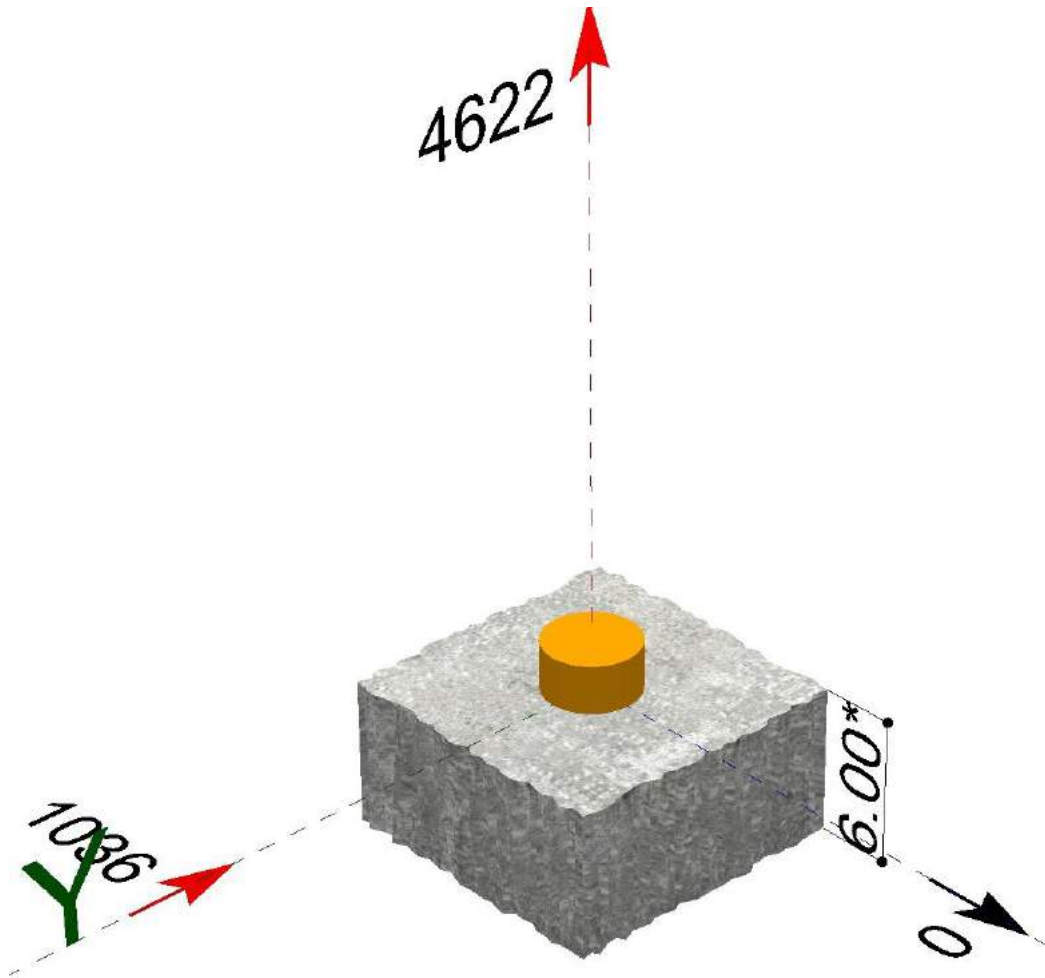
FREEZER UNIT

SUPPORTING NORMAL WEIGHT CONC. SLAB W/ MIN. f'c=4000psi & MIN. 6" THICKNESS BY OTHERS

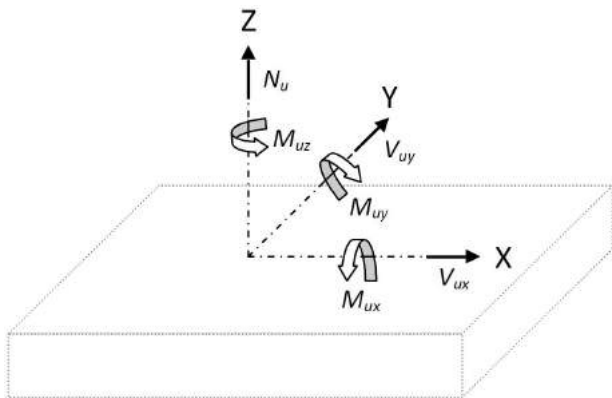


**3** SIDE ELEVATION VIEW

**GEOMETRY:**




**LOAD ACTIONS: [lb], [ft-lb]**



Design loads / actions		
$N_u$	4622	lb
$V_{ux}$	0	lb
$V_{uy}$	1036	lb
$M_{ux}$	0	ft-lb
$M_{uy}$	0	ft-lb
$M_{uz}$	0	ft-lb

Load reversal	100	%
X-direction		
Load reversal	100	%
Y-direction		

	Company name:	Date:	7/14/2015
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**SUMMARY:**

Selected anchor: Power-Stud+ SD4  
 3/4" Ø; hnom 4-1/2" (114mm), Grade 304 (stainless)

Effective embedment depth:  $h_{ef} = 3.750$  inch

Approval: ESR-2502  
 Issued: 5/1/2014



<b>Basic principles of Design:</b>			
Design method:	ACI 318-11 (Appendix D)		
Concrete:	Normal weight concrete	cracked concrete	$f'_c = 4000$ psi
Load combination:	taken from Section 9.2		
	Factored loads		
	$\Omega =$ User enters load		
Anchor Parameters:	$c_{min} = 5.00$ inch	$s_{min} = 9.00$ inch	$h_{min} = 6.00$ inch
	$c_{ac} = 9.00$ inch	$s_{cr} = 11.25$ inch	Anchor Ductility: Yes
Reinforcement:	none edge reinforcement or < #4 bar		
	Tension: Condition B	Shear: Condition B	
Stand-off:	not existent		
Seismic Loads:	Yes		
	Tension load	Yes (D.3.3.4.3(d))	
	Shear load	Yes (D.3.3.5.3(c))	

**Resulting anchor forces / load distribution::**

Anchor No.	Tension load	Shear load
#1	4622 lb	1036 lb
Maximum	4622 lb	1036 lb

Max. concrete compression strain: 0.00 ‰

Max. concrete compression stress: 0 psi

Resulting tension force: 4622 lb

Resulting compression force: 0 lb

Calculations:	Design proof:	Demand	Capacity	Status
	Tension load	4622 lb	4702 lb	$0.98 \leq 1.0$
	Shear load	1036 lb	5034 lb	$0.21 \leq 1.0$
	Interaction	-	-	$0.99 \leq 1.0$
				OK

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines and must be checked for plausibility.

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Company name:

Project:

Date: 7/14/2015

Version: 2.2.5543.30490

Project number:

Page: 4/6

**DESIGN PROOF TENSION LOADING:****Reference****Steel strength:**

$$N_{sa} = 21380 \text{ lb} \quad \text{D.5.1}$$

$$\Phi * N_{sa} = \Phi * N_{sa} \quad \text{D.5.1.2}$$

$$= 0.75 * 21380 \text{ lb} = 16035 \text{ lb}$$

$$N_{ua} = 4622 \text{ lb}$$

$$\text{Design proof: } N_{ua} / (\Phi * N_{sa}) = 4622 \text{ lb} / 16035 \text{ lb} = 0.29 \leq 1.00$$

**Concrete Breakout Strength:**

$$h_{ef} = 3.750 \text{ inch}$$

$$k_c = 21.0$$

$$N_b = k_c * f'_c{}^{0.5} * \lambda_a * h_{ef}{}^{1.5} \quad \text{D.5.2.2}$$

$$= 21.0 * 63.25 * 1.00 * 7.262 = 9645 \text{ lb}$$

$$A_{Nc0} = 126.56 \text{ inch}^2$$

$$A_{Nc} = 126.56 \text{ inch}^2$$

$$\psi_{ed,N} = 1.000 \quad \text{D.5.2.5}$$

$$\psi_{c,N} = 1.00 \quad \text{D.5.2.6}$$

$$c_{ac} = 9.00 \text{ inch}$$

$$\psi_{cp,N} = 1.000 \quad \text{D.5.2.7}$$

$$\Phi * N_{cb} = \Phi_{seis} * \Phi * (A_{Nc} / A_{Nc0}) * \psi_{ed,N} * \psi_{c,N} * \psi_{cp,N} * N_b \quad \text{D.5.2.1}$$

$$= 0.75 * 0.65 * (126.56 / 126.56) * 1.000 * 1.00 * 1.000 * 9645 \text{ lb}$$

$$= 4702 \text{ lb}$$

$$N_{ua} = 4622 \text{ lb}$$

$$\text{Design proof: } N_{ua} / (\Phi * N_{cb}) = 4622 \text{ lb} / 4702 \text{ lb} = 0.98 \leq 1.00$$

**DESIGN PROOF SHEAR LOADING:****Reference****Steel strength (without lever arm):**

$$V_{sa,eq} = 7745 \text{ lb} \quad \text{D.6.1}$$

$$\Phi * V_{sa,eq} = \Phi * V_{sa,eq} \quad \text{D.6.1.2}$$


$$= 0.65 * 7745 \text{ lb} = 5034 \text{ lb}$$

$$V_{ua} = 1036 \text{ lb}$$

$$\text{Design proof: } V_{ua} / (\Phi * V_{sa,eq}) = 1036 \text{ lb} / 5034 \text{ lb} = 0.21 \leq 1.00$$

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines and must be checked for plausibility.

[www.powers.com](http://www.powers.com) - Powers Fasteners (see website for regional contact information).

	Company name:	Date: 7/14/2015
	Project:	Page: 5/6
	Version: 2.2.5543.30490	Project number:

**Pryout strength:**

$$\begin{aligned}
 h_{ef} &= 3.750 && \text{inch} \\
 k_C &= 21.0 \\
 N_b &= k_C * f'_c{}^{0.5} * \lambda_a * h_{ef}{}^{1.5} && \text{D.5.2.2} \\
 &= 21.0 * 4000^{0.5} * 1.00 * 3.750^{1.5} = 9645 \text{ lb} \\
 A_{Nc} &= 126.56 && \text{inch}^2 \\
 A_{Nc0} &= 126.56 && \text{inch}^2 \\
 \psi_{ed,N} &= 1.000 && \text{D.5.2.5} \\
 \psi_{cp,N} &= 1.000 && \text{D.5.2.7} \\
 \psi_{c,N} &= 1.000 && \text{D.5.2.6} \\
 k_{cp} &= 2.0 && \text{D.6.3.1} \\
 \Phi * V_{cp} &= \Phi_{seis} * \Phi * (A_{Nc} / A_{Nc0}) * \psi_{ed,N} * \psi_{cp,N} * \psi_{c,N} * N_b * k_{cp} \\
 &= 1.0 * 0.70 * (126.56 / 126.56) * 1.000 * 1.000 * 1.000 * 9645 \text{ lb} * 2.0 \\
 &= 13503 && \text{lb} \\
 V_{ua} &= 1036 && \text{lb} \\
 \text{Design proof: } &V_{ua} / (\Phi * V_{cp}) = 1036 \text{ lb} / 13503 \text{ lb} = 0.08 \leq 1.00
 \end{aligned}$$

**COMBINATION TENSION / SHEAR LOAD:****Reference**


Interaction:

$$\begin{aligned}
 \text{Design proof: } &= N_u / (\Phi * N_n) + V_u / (\Phi * V_n) / 1.2 && \text{D.7.3} \\
 &= (0.98 + 0.21) / 1.2 = 0.99 \leq 1.0
 \end{aligned}$$

**Fastening ok!**

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines and must be checked for plausibility.

[www.powers.com](http://www.powers.com) - Powers Fasteners (see website for regional contact information).

	Company name:		Date:	7/14/2015
	Project:		Page:	6/6
	Version: 2.2.5543.30490	Project number:		

### **WARNINGS / REMARKS:**

Calculations including seismic design requirements in accordance with ACI 318 D.3.3 are required for anchors in structures assigned to seismic design categories C, D, E and F. Under these seismic conditions, the direction of shear may not be predictable. As default and in accordance with ACI 318 D.3.3 the full shear force is assumed also in reverse direction for a safe design. This may influence the direction of the controlling concrete breakout strength.

#### D.3.3.4.3 (c) Selected:

Anchor or anchor group shall be designed for the max design load combinations that include E, with E increased by  $\Omega_o$ . The anchor design tensile strength shall satisfy the tensile strength requirements of D.3.3.4.4.

Per ACI 318-11, Part D.3.3.4.3(b) the anchor or group of anchors shall be designed for the maximum tension that can be transmitted to the anchor or group of anchors based on the development of a ductile yield mechanism in the attachment in flexure, shear, or bearing, or a combination of those conditions, and considering both material overstrength and strain hardening effects for the attachment. The anchor design tensile strength shall be calculated from D.3.3.4.4. Per ACI 318-11, Part D.3.3.4.3(c) the anchor or group of anchors shall be designed for the maximum tension that can be transmitted to the anchors by a non-yielding attachment. The anchor design tensile strength shall be calculated from D.3.3.4.4. Per ACI 318-11, Part D.3.3.4.3(d) The anchor or group of anchors shall be designed for the maximum tension obtained from design load combinations that include E, with E increased by  $\Omega_o$ . The anchor design tensile strength shall satisfy the tensile strength requirements of D.4.1.1.

Per ACI 318-11, Part D.3.3.5.3(a) the anchor or group of anchors shall be designed for the maximum shear that can be transmitted to the anchor or group of anchors based on the development of a ductile yield mechanism in the attachment in flexure, shear, or bearing, or a combination of those conditions, and considering both material overstrength and strain hardening effects for the attachment. Per ACI 318-11, Part D.3.3.5.3(b) the anchor or group of anchors shall be designed for the maximum shear that can be transmitted to the anchors by a non-yielding attachment. Per ACI 318-11, Part D.3.3.5.3(c) the anchor or group of anchors shall be designed for the maximum shear obtained from design load combinations that include E, with E increased by  $\Omega_o$ . The anchor design shear strength shall satisfy the shear strength requirements of D.4.1.1.

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines and must be checked for plausibility.

[www.powers.com](http://www.powers.com) - Powers Fasteners (see website for regional contact information).

Project: Lab Freezer Restraints (Thermo Fisher)  
 Project No.: 0932  
 Subject: Angle Bracket for Shear Stop

Designer: JY  
 Date: 7/14/2015

## Check of Angle Bracket

Max shear force to single L2-1/2x2x1/4  $V_{\max} := 1036\text{ lbf}$

For L2-1/2x2x1/4  $k := 0.5\text{ in}$

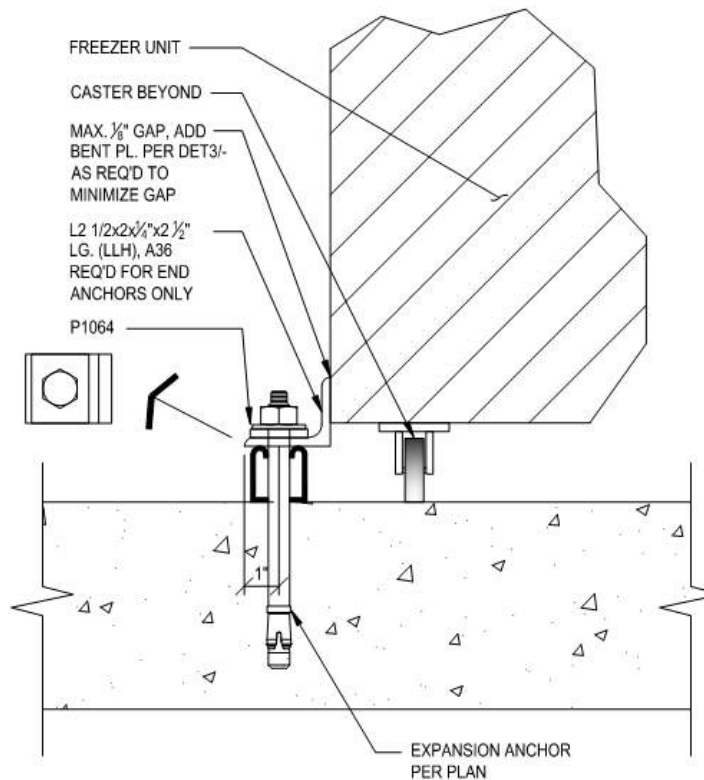
Assumign height of bearing area to be 0.75 in  $h := 0.75\text{ in}$

$$M_u := V_{\max} \cdot \left( 2\text{ in} - k - \frac{h}{2} \right) = 1.165 \cdot \text{kip} \cdot \text{in}$$

$$\phi M_n := 0.9 \cdot 36\text{ ksi} \cdot 2.5\text{ in} \cdot \frac{(0.25\text{ in})^2}{4} = 1.266 \cdot \text{kip} \cdot \text{in}$$

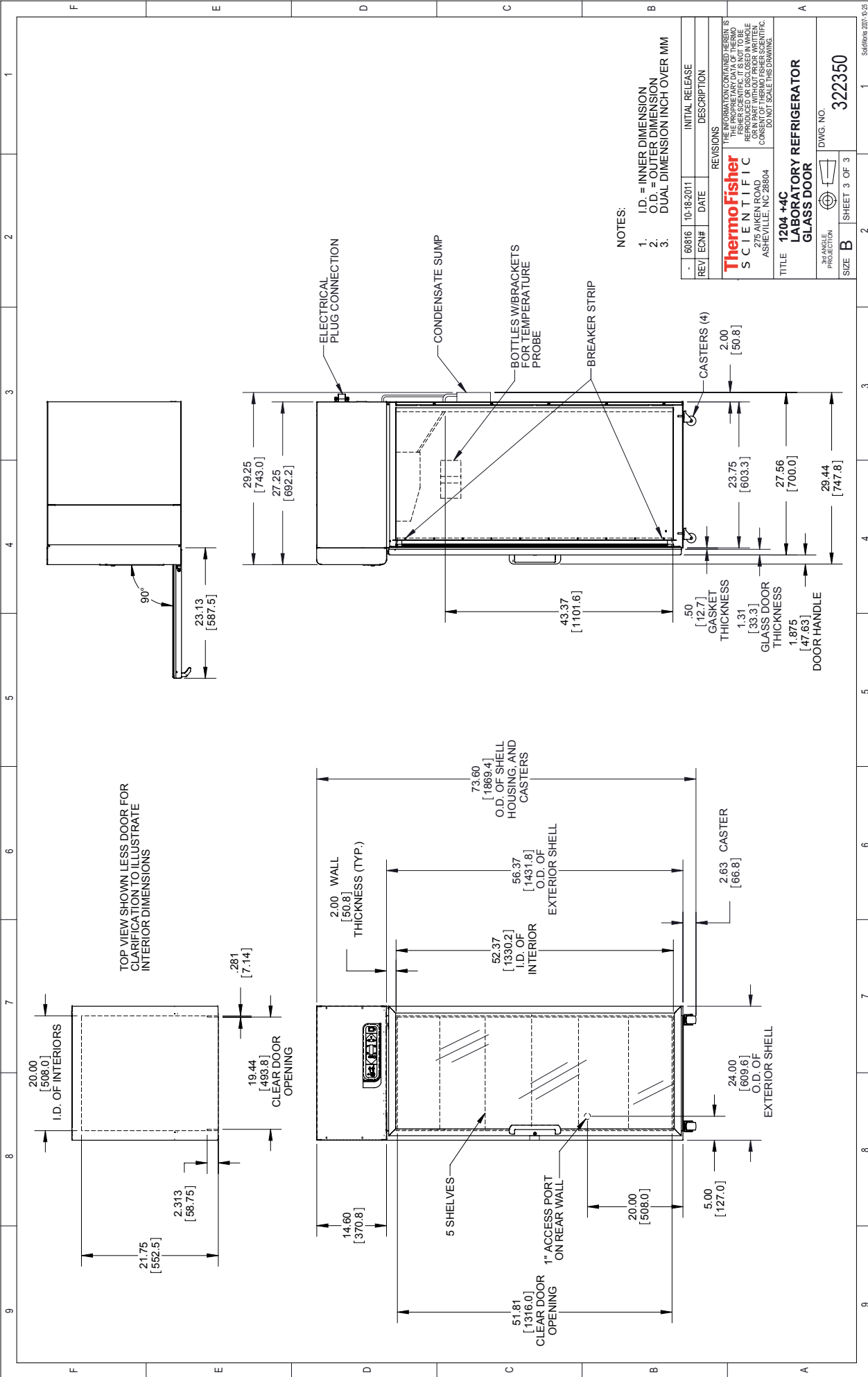
$$\text{DCR} := \frac{M_u}{\phi M_n} = 0.921$$

Check := if(DCR < 1, "O.K.", "N.G.") = "O.K."



---

## **Appendix A. Cut Sheets of ThermoFisher Scientific Freezers**



REV	CONF	DATE	INITIAL	RELEASE	DESCRIPTION
-	60816	10-18-2011			

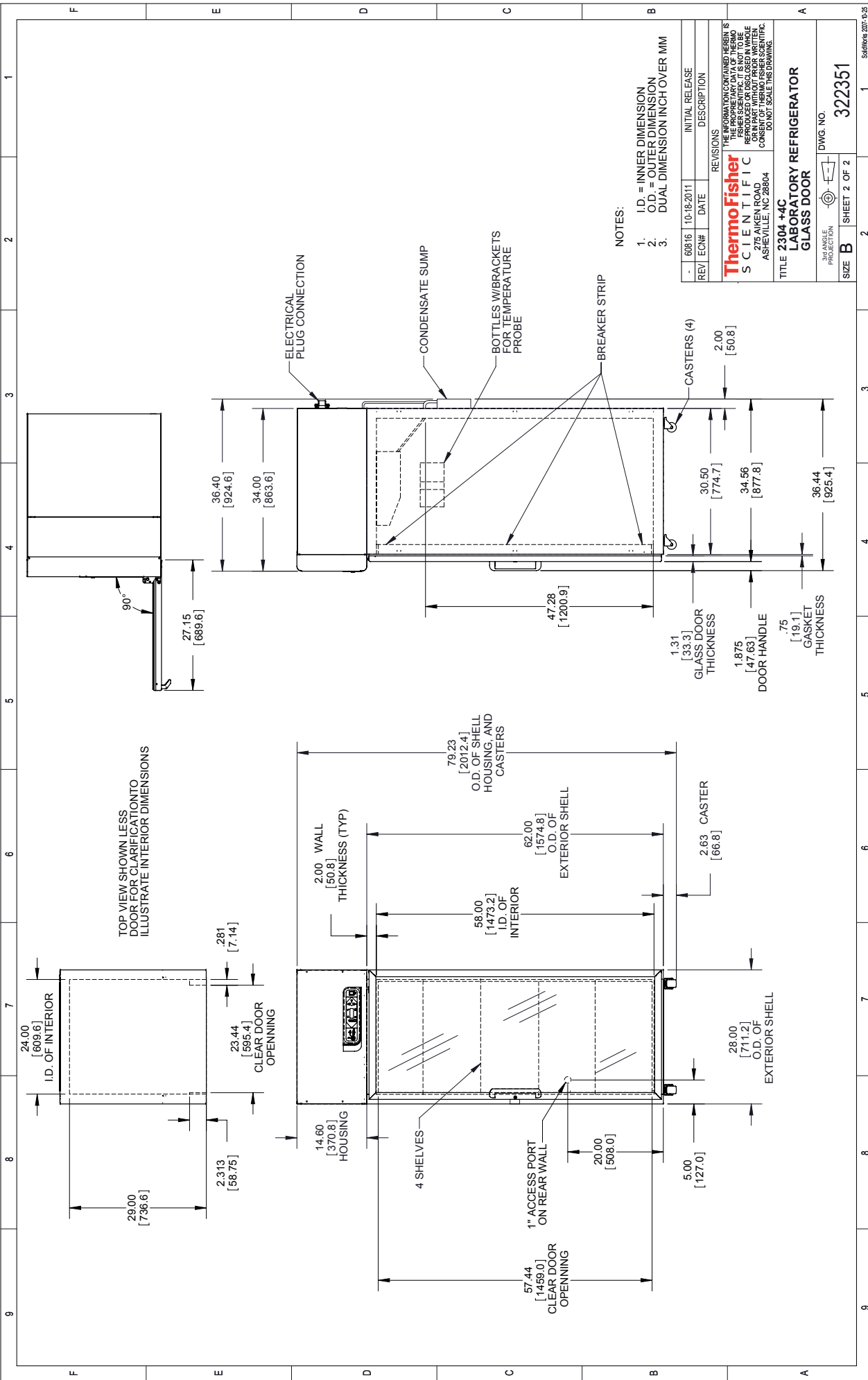
  

REVISIONS	
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<b>ThermoFisher</b> <b>SCIENTIFIC</b> 7700 AVENUE ROAD ASTEVILLE, NC 28804
<b>TITLE</b> 1204 +4C LABORATORY REFRIGERATOR GLASS DOOR
<b>DWG. NO.</b> 322350
<b>SIZE</b> B <b>SHEET</b> 3 OF 3

S:\dwr\1204\1204.dwg 2011-10-18 10:25



NOTES:

1. I.D. = INNER DIMENSION
2. O.D. = OUTER DIMENSION
3. DUAL DIMENSION INCH OVER MM

REV	ENGR	DATE	INITIAL RELEASE	DESCRIPTION
-	60816	10-18-2011		

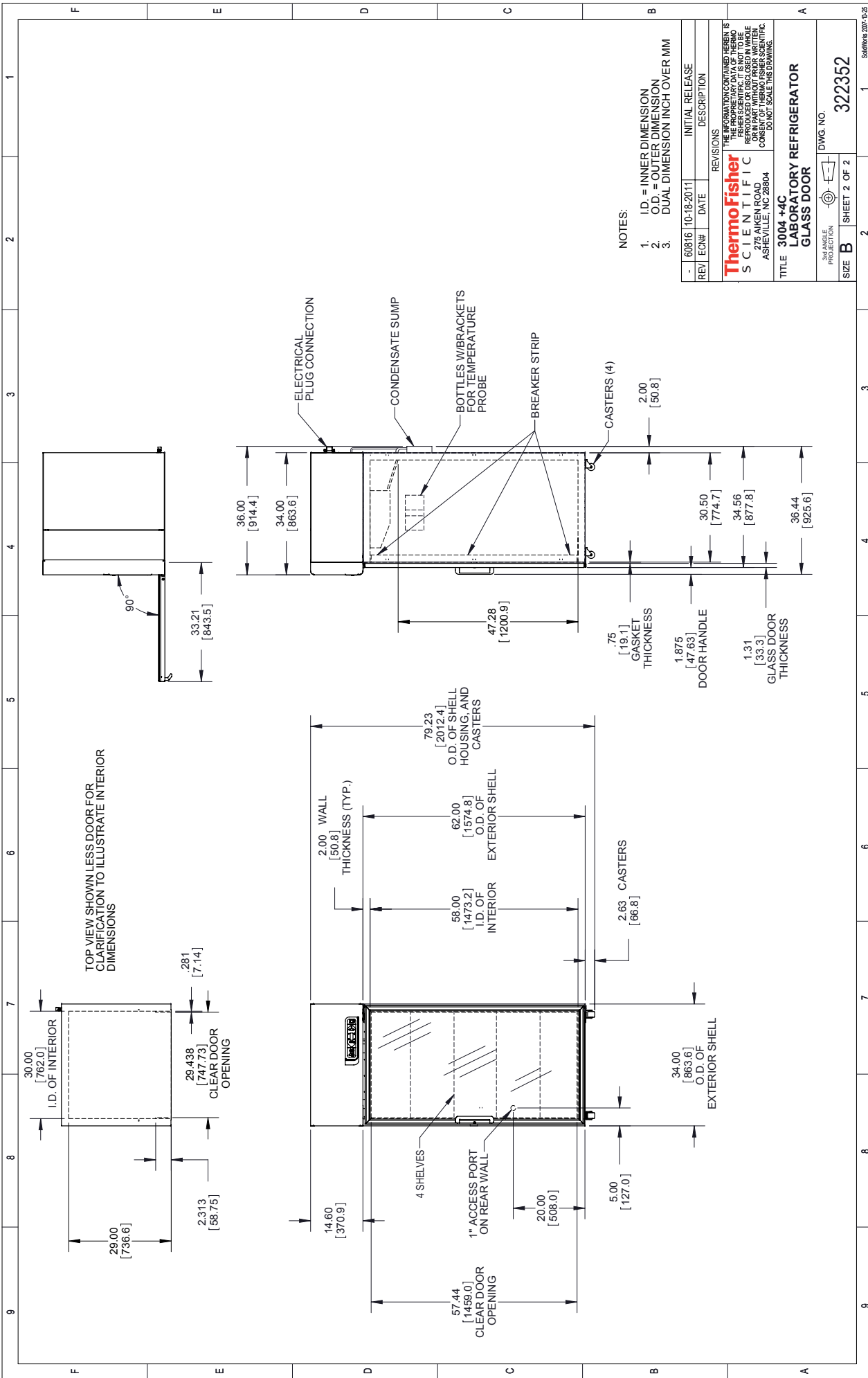
**ThermoFisher**  
**SCIENTIFIC**  
 700 ALLEN WAY  
 ASHEVILLE, NC 28804

TITLE **2304 +4C**  
**LABORATORY REFRIGERATOR**  
**GLASS DOOR**

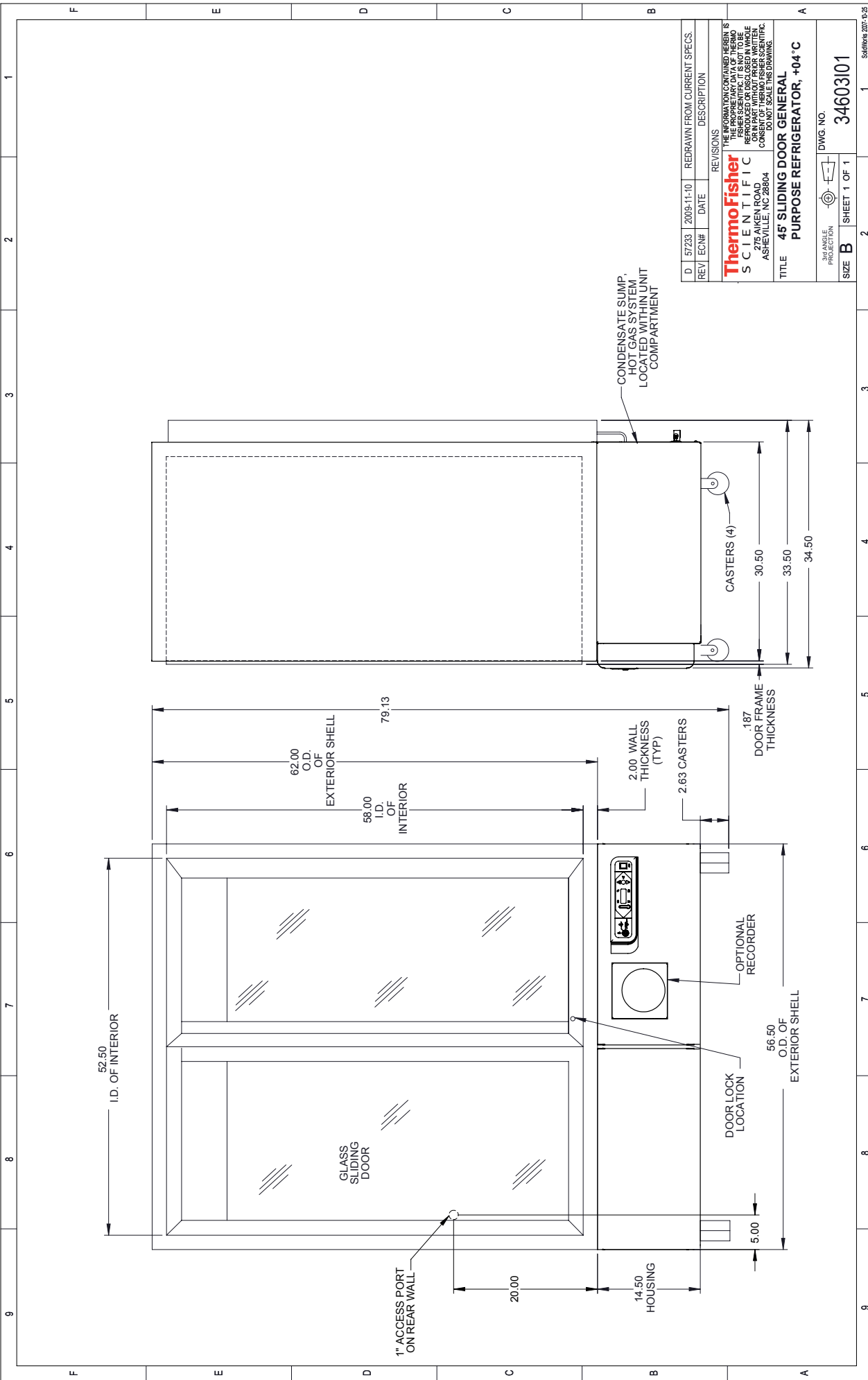
SCALE: AS SHOWN  
 PROJECTION:   
 DWG. NO. **322351**

SHEET 2 OF 2

Address: 2007-03-25







REV	ECNF	DATE	DESCRIPTION
D	57233	2009-11-10	REDRAWN FROM CURRENT SPECS.

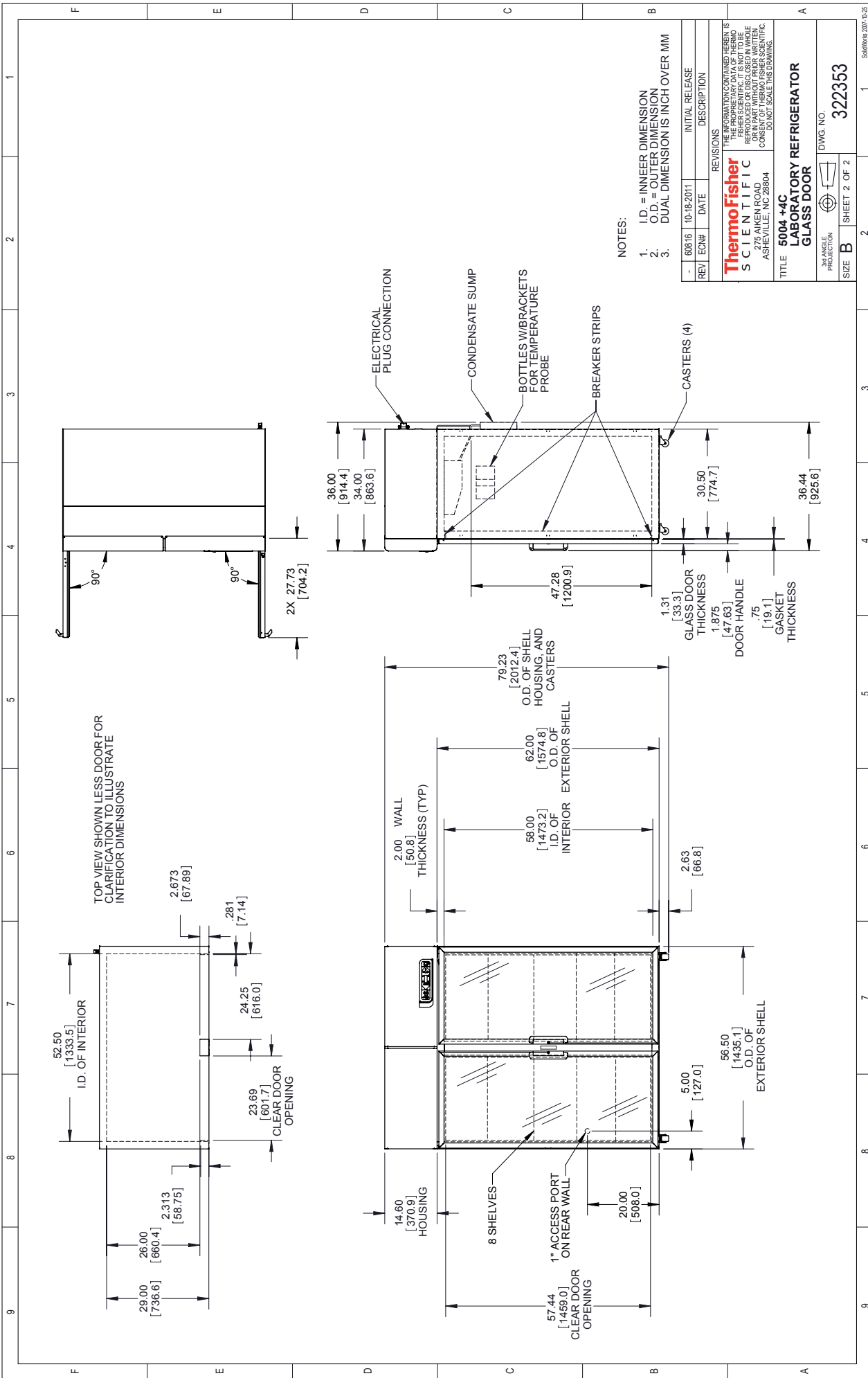
  

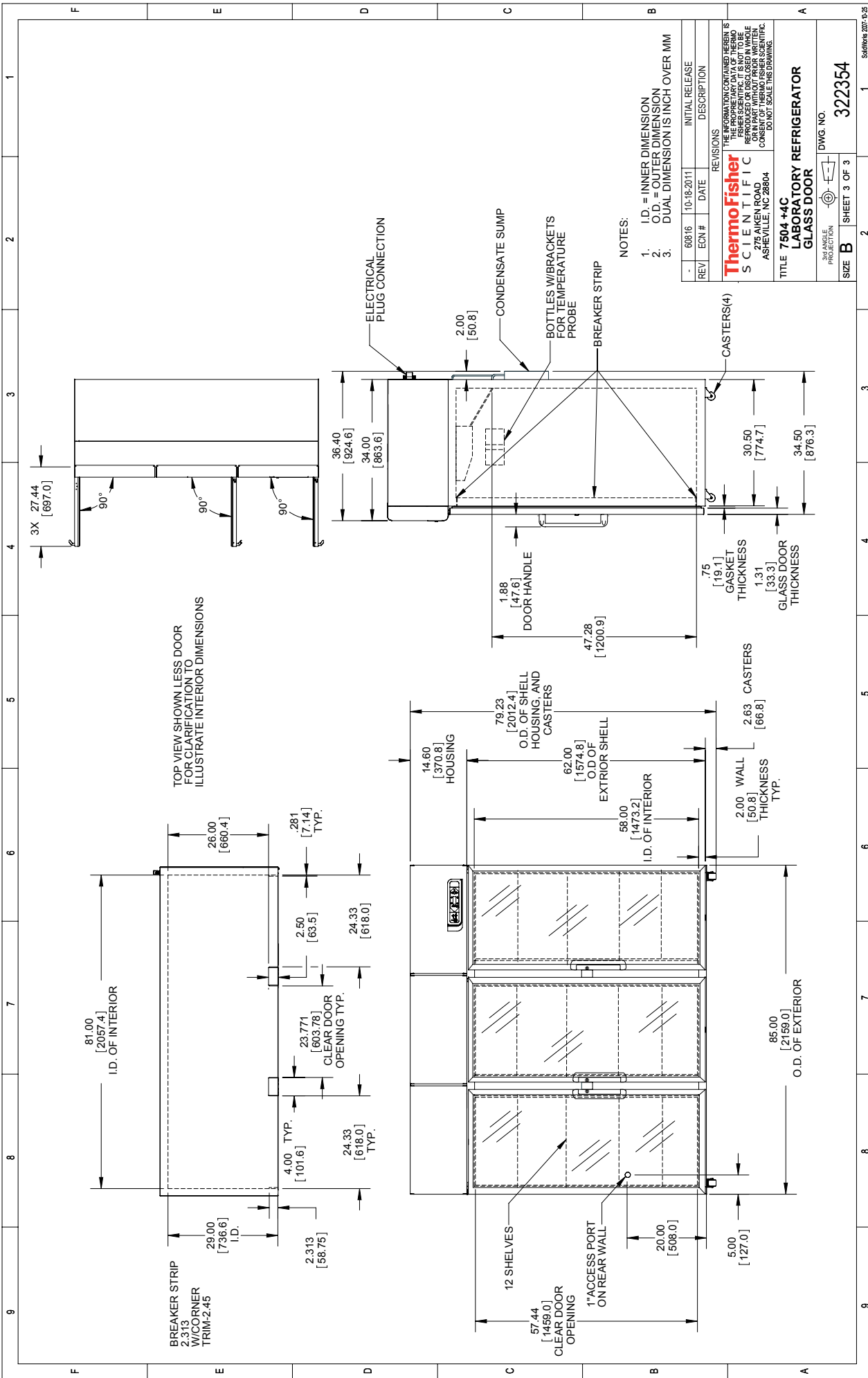
REVISIONS	
<b>ThermoFisher</b>	THE INFORMATION CONTAINED HEREIN IS THE PROPERTY OF THERMO FISHER SCIENTIFIC. IT IS NOT TO BE REPRODUCED OR DISCLOSED IN ANY MANNER WITHOUT THE WRITTEN CONSENT OF THERMO FISHER SCIENTIFIC. DO NOT SCALE THIS DRAWING.
<b>SCIENTIFIC</b>	
ASTEVILLE, NC 28804	

TITLE	<b>45° SLIDING DOOR GENERATOR, +04 °C</b>
DWG. NO.	<b>34603101</b>
SIZE	<b>B</b>
SHEET	<b>1 OF 1</b>

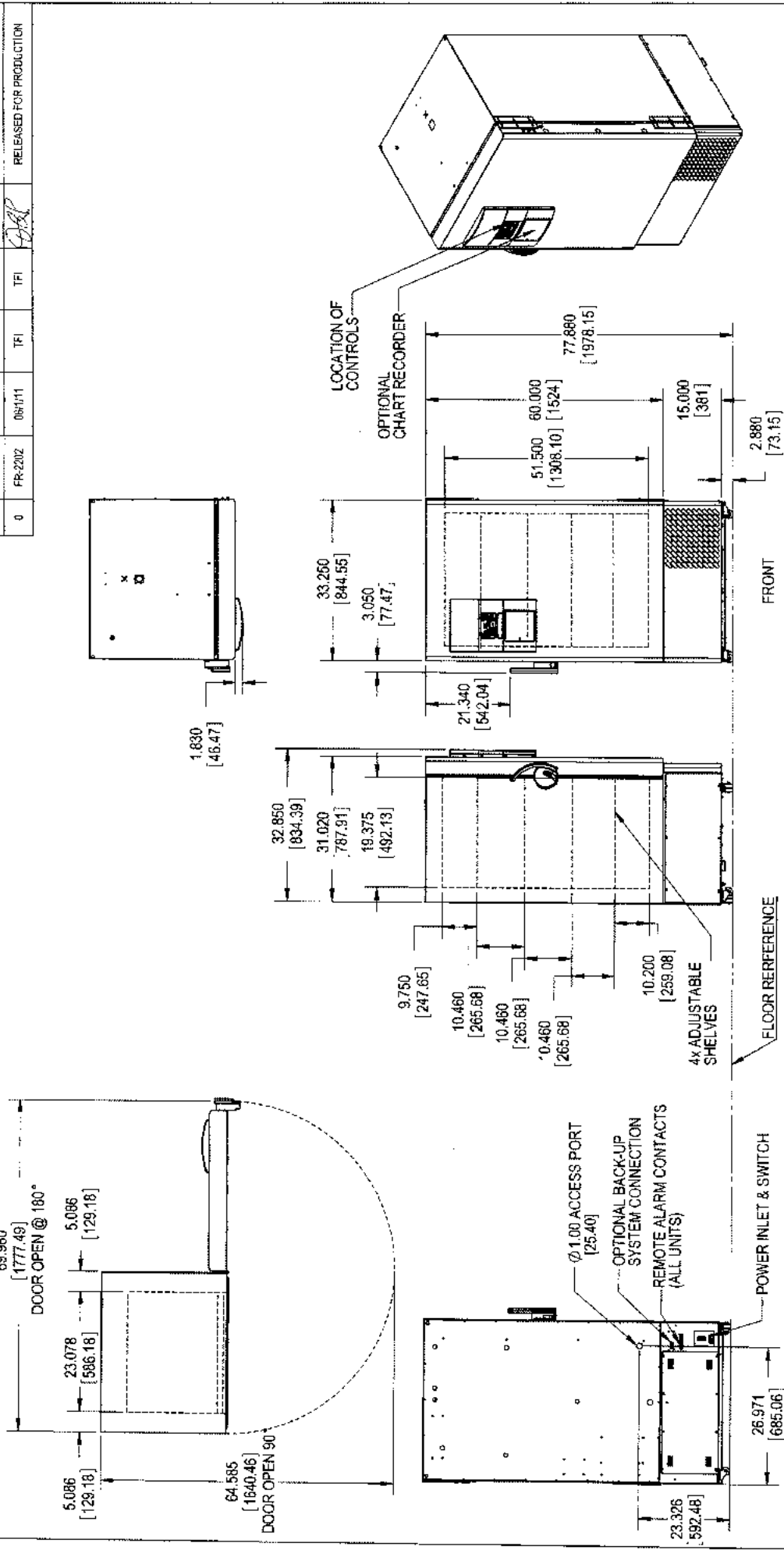
ADDRESS: 2007-0-25  
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 2  
 3  
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 8  
 9





DRAWING NUMBER: 8920-00-1

REV	ECN NO.	DATE	BY	CAD	APPD	DESCRIPTION OF REVISION
0	FR-2202	06/11/11	TFI	TFI	SPH	RELEASED FOR PRODUCTION



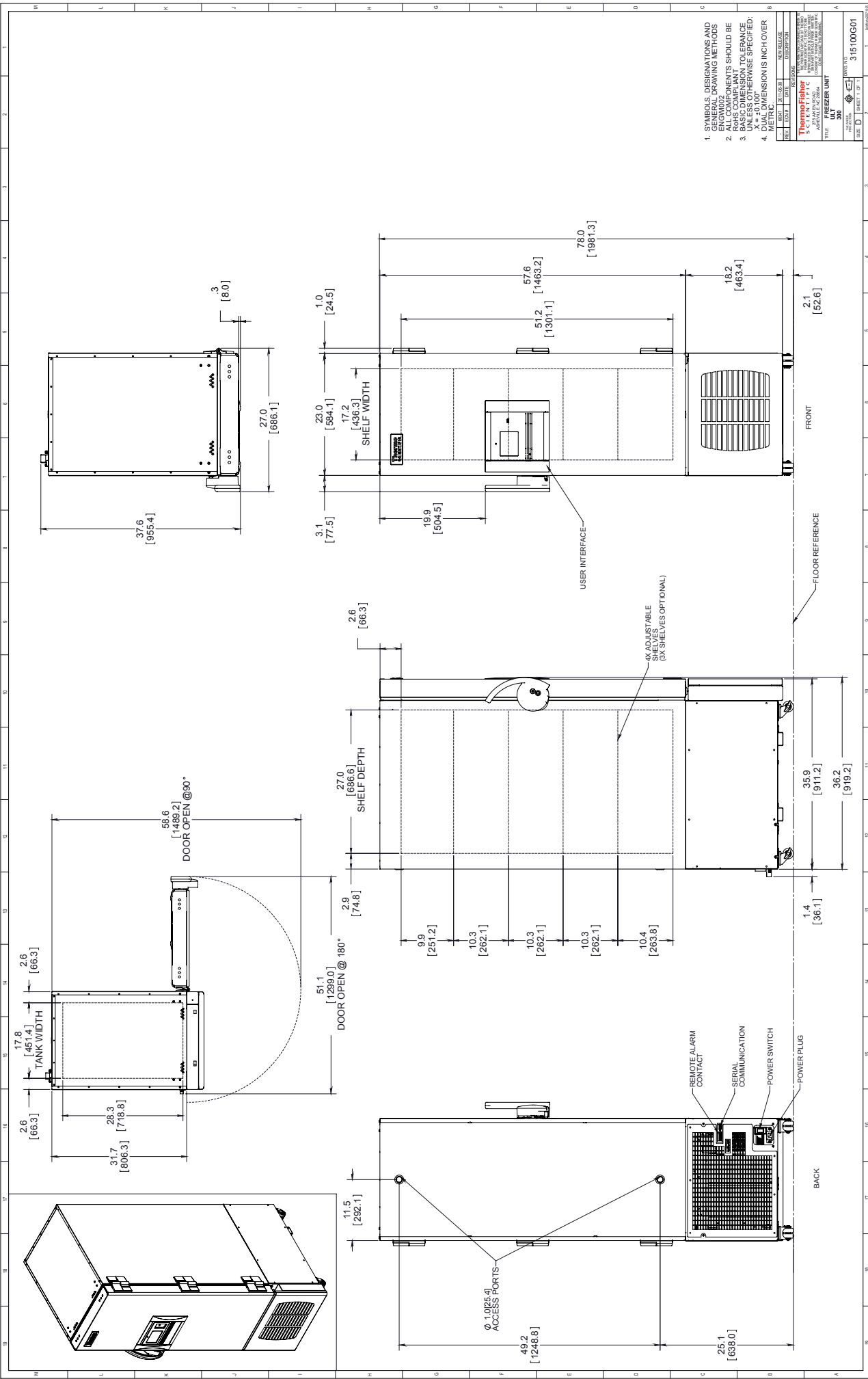
THIS DOCUMENT CONTAINS PROPRIETARY INFORMATION AND SUCH INFORMATION IS NOT TO BE DISCLOSED TO OTHERS FOR ANY PURPOSE NOR USED FOR MANUFACTURING PURPOSES WITHOUT WRITTEN PERMISSION FROM THERMO FISHER SCIENTIFIC

MODEL/PART NAME: 13 CU FT. -86°C UPRIGHT FREEZERS  
 GENERAL DIMENSION DRAWING U/LT  
 DWG. TFI CAD TFI APPD. SSK DATE: 06/11/2011 SCALE: N/A

MATERIAL: N/A	PAINT COLOR: N/A	TOLERANCE UNLESS OTHERWISE SPECIFIED: .0005 ± .00	ANGLES: DECIMAL .0005 ± .00
<b>ThermoFisher</b> <b>SCIENTIFIC</b> BOX 649, MARIETTA, OHIO 45750			DRAWING NUMBER <b>8920-00-1</b> SIZE <b>B</b>

NOTE: DUAL DIMENSION IS INCH OVER METRIC.

8920-00-1-2

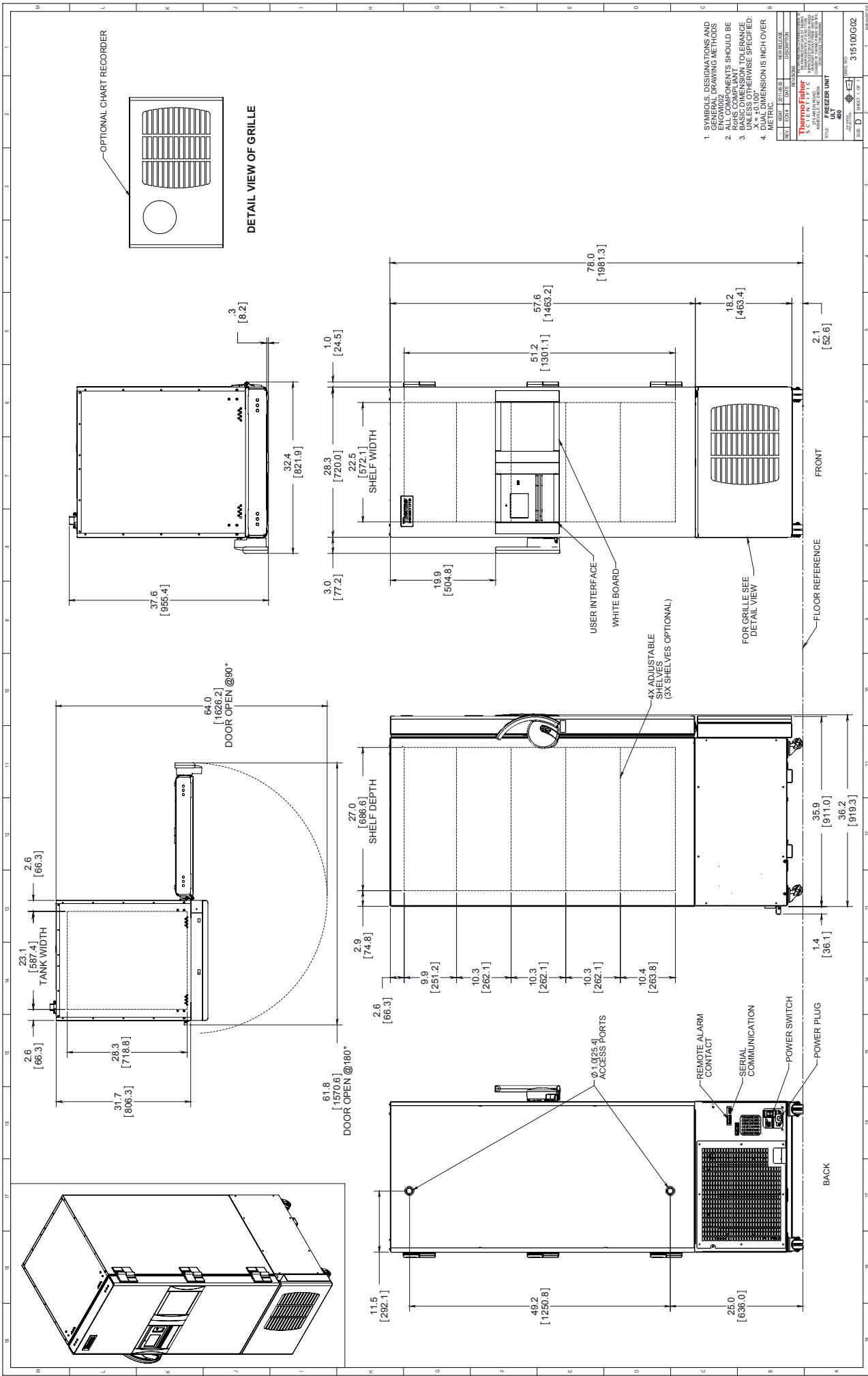


1. SYMBOLS, DESIGNATIONS AND GENERAL DRAWING METHODS  
 2. DIMENSIONS SHOULD BE IN INCHES UNLESS OTHERWISE SPECIFIED  
 3. UNLESS OTHERWISE SPECIFIED, X = ±0.100"  
 4. METRIC DIMENSIONS IN INCH OVER

REV	DATE	DESCRIPTION
1	01/10/01	REVISION
2	01/10/01	REVISION

**ThermoFisher**  
 315100G01  
 FREEZER UNIT

UNIT: FREEZER UNIT  
 PART NO: 315100G01



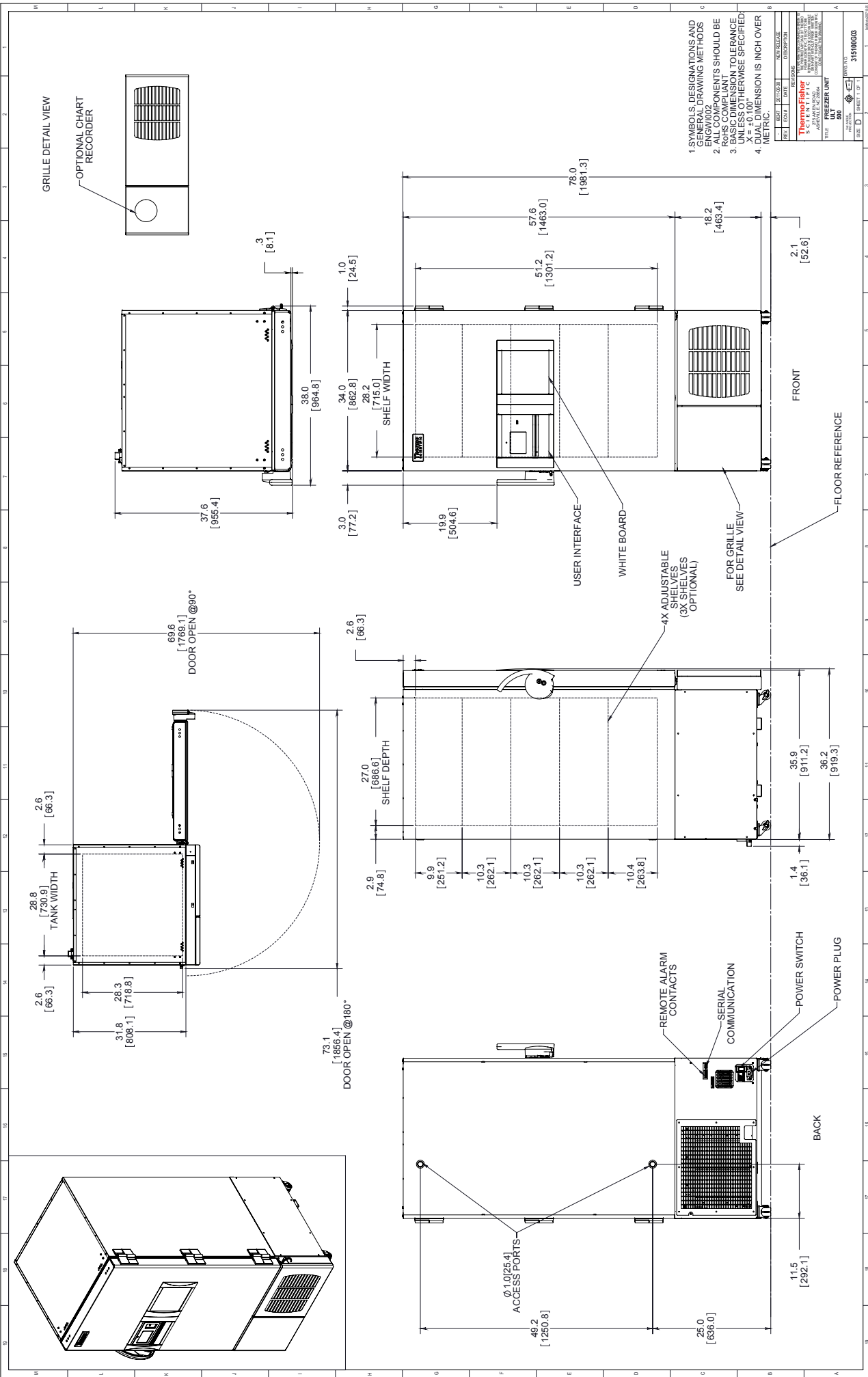
1. SYMBOLS, DESIGNATIONS AND GENERAL DRAWING METHODS
2. ALL COMPONENTS SHOULD BE ROLS COMPLIANT TO ENANCE UNLESS OTHERWISE SPECIFIED
3. X = ±0.10"
4. METRIC DIMENSIONS IS INCH OVER

REV	DATE	DESCRIPTION
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2	01/10/01	REVISED
3	01/10/01	REVISED
4	01/10/01	REVISED
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97	01/10/01	REVISED
98	01/10/01	REVISED
99	01/10/01	REVISED
100	01/10/01	REVISED

**ThermoFisher**  
 5000 CENTRAL EXPRESSWAY  
 FORT WORTH, TEXAS 76102  
 TEL: 817.254.1000 FAX: 817.254.1001  
 WWW.THERMOFISHER.COM

UNIT: **FREEZER UNIT**

SIZE: **D** SHEET: **1** OF **3** PART NO: **315100G02**



1. SYMBOLS, DESIGNATIONS AND GENERAL DRAWING METHODS SHALL BE IN ACCORDANCE WITH THE INTERNATIONAL STANDARDS.
2. ALL COMPONENTS SHOULD BE ROHS COMPLIANT.
3. BASIC DIMENSION TOLERANCE IS ±0.10, UNLESS OTHERWISE SPECIFIED.
4. DUAL DIMENSION IS INCH OVER METRIC.

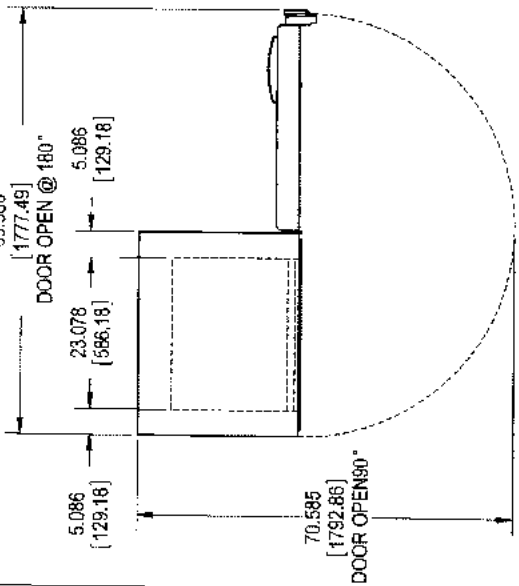
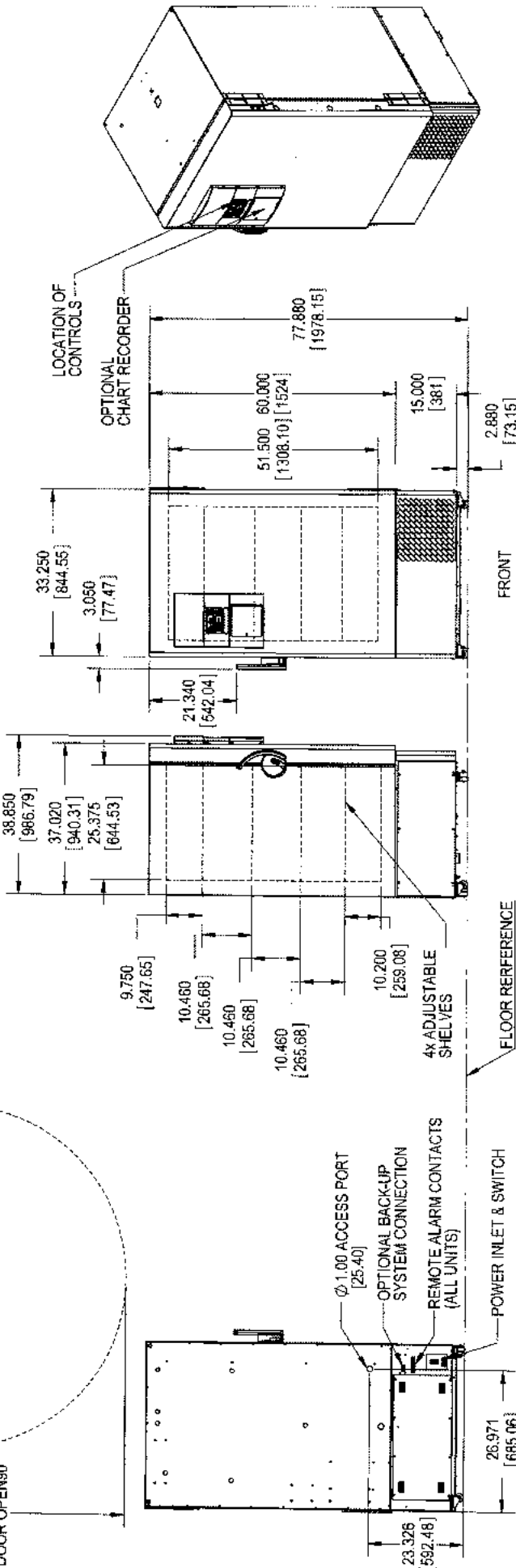
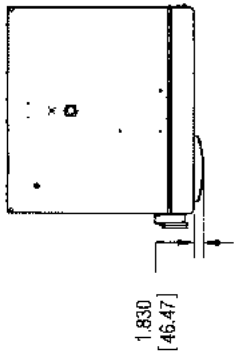
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ThermoFisher  
 5000 UNIVERSITY AVENUE  
 WASHINGTON, DC 20004  
 TEL: 1-800-762-4229  
 FAX: 1-800-762-4229  
 WWW.THERMOFISHER.COM

UNIT: FREEZER UNIT  
 MODEL: 31510003  
 SHEET 1 OF 1

DRAWING NUMBER: 8924-00-1

REV	ECN NO.	DATE	BY	CAD	APPD	DESCRIPTION OF REVISION
0	FR-7709	08/11/11	TFI	TFI	[Signature]	RELEASED FOR PRODUCTION



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MODEL PART NAME: 17 CU FT., -85°C UPRIGHT FREEZERS  
 GENERAL DIMENSION DRAWING U.T.  
 DWG. TFI | CAC. TFI | APPD. SSK | DATE: 08/11/2011 | SCALE: N/A

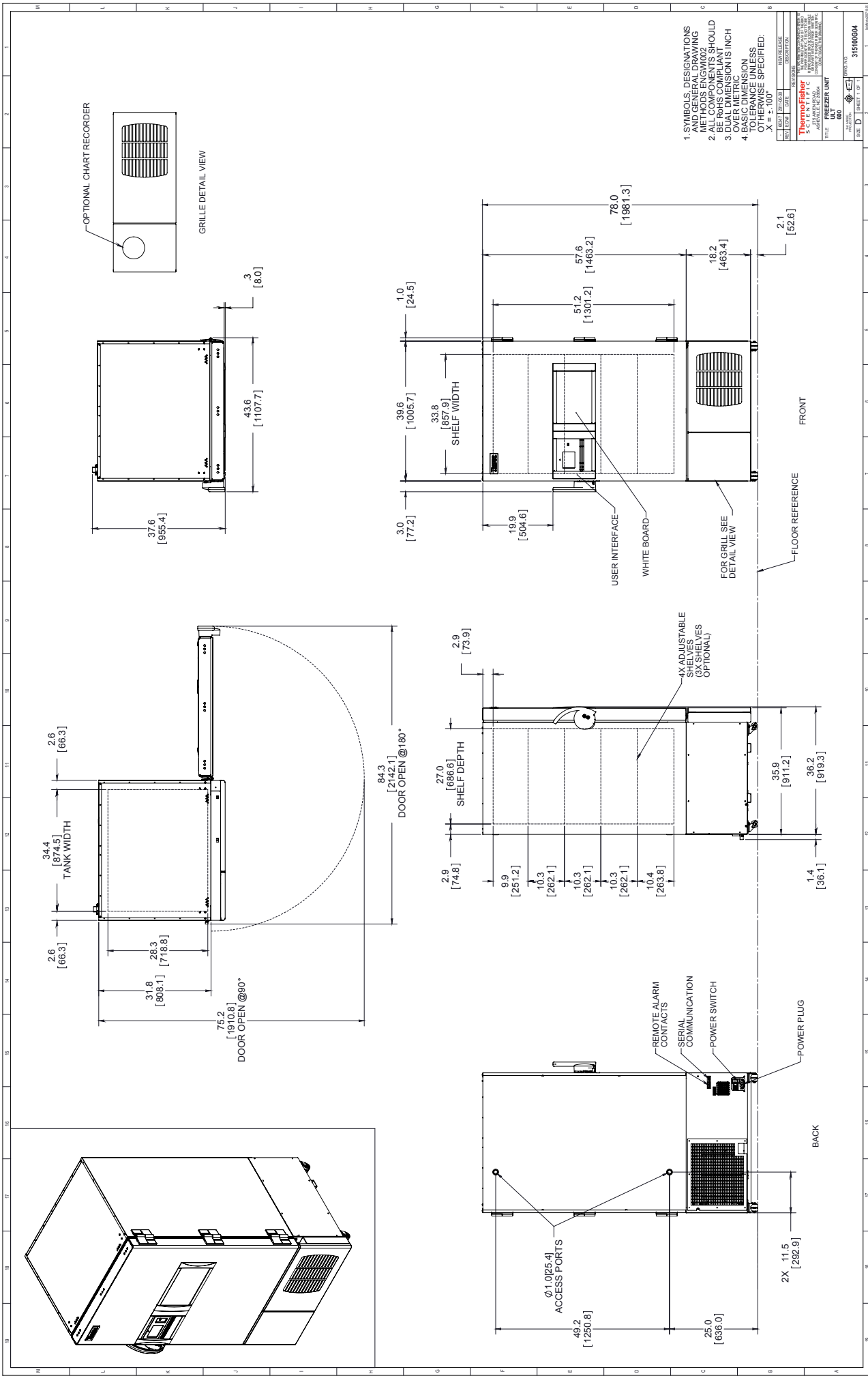
**ThermoFisher**  
**SCIENTIFIC**  
 BOX 648, MARIETTA, OHIO 45750

NOTE: DUAL DIMENSION IS INCH OVER METRIC.

TOLERANCE UNLESS OTHERWISE SPECIFIED:	DRAWING NUMBER
ANGLES: X0°±.06	8924-00-1
X00°±.08	SIZE
X000°±.08	B

8924-00-1-33





1. SYMBOLS, DESIGNATIONS AND GENERAL DRAWING METHODS ENG1002
2. ALL COMPONENTS SHOULD BE DRAWN TO THE UNIT DIMENSION UNLESS OTHERWISE SPECIFIED.
3. DIMENSION IS INCH OVER METRIC
4. BASIC DIMENSION IS OVER DIMENSION UNLESS OTHERWISE SPECIFIED.
5. X = 1/100'

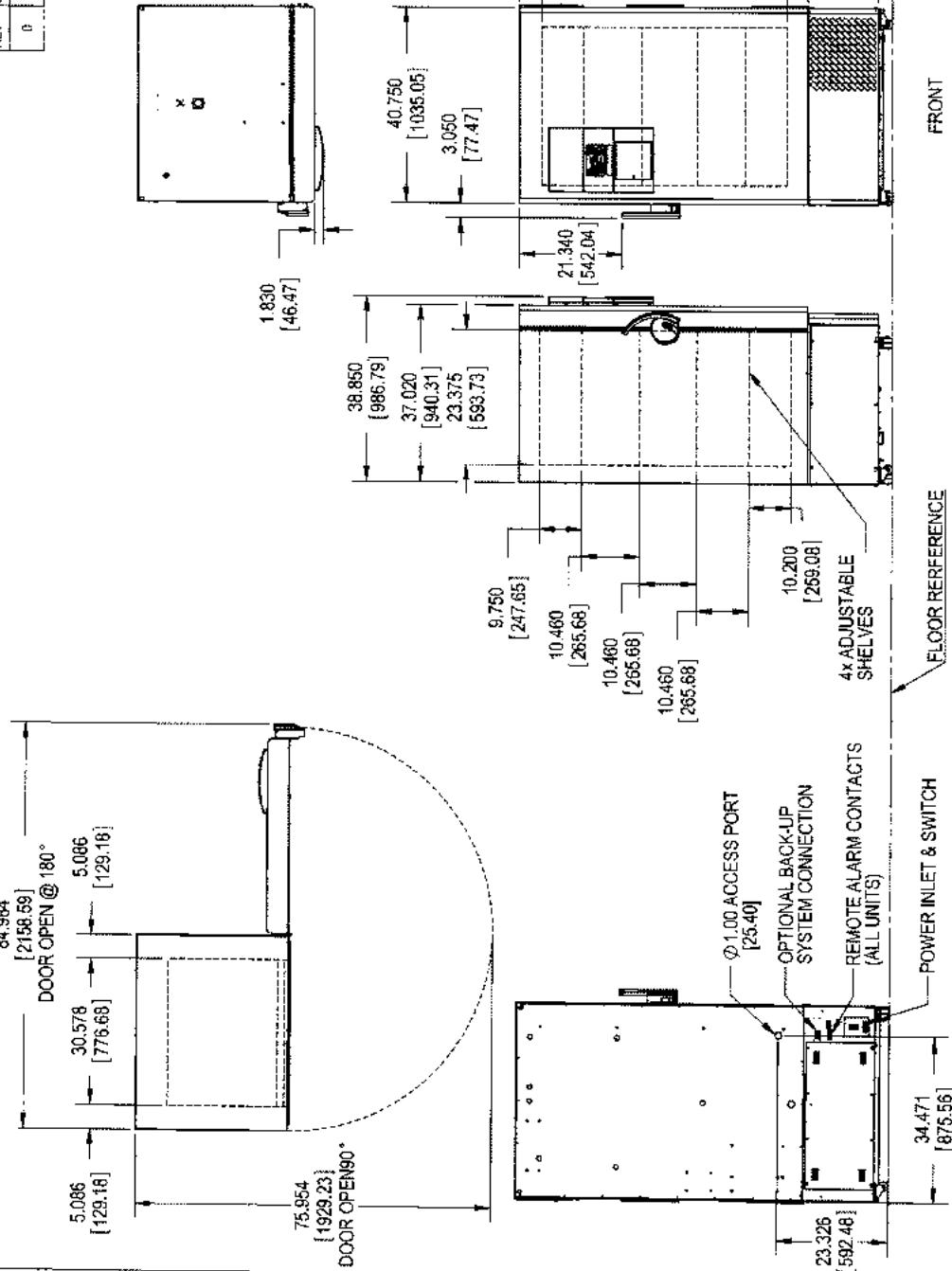
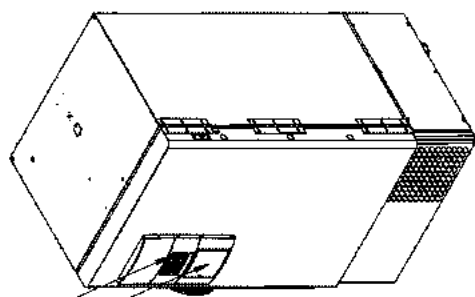
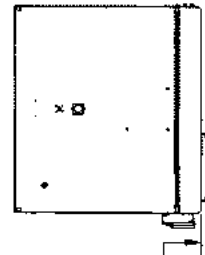
REV	DATE	DESCRIPTION
1	01/10/01	ISSUE FOR RELEASE
2	01/10/01	REVISED

**ThermoFisher**  
 3500 CENTRE EXCHANGE DRIVE  
 WALTHAM, MA 01981  
 TEL: 781.942.1000 FAX: 781.942.1001  
 WWW.THERMOFISHER.COM

UNIT: FREEZER UNIT  
 MODEL: 315100004  
 SIZE: D SHEET 1 OF 1

DRAWING NUMBER: 8926-00-1

REV	ECN NO.	DATE	BY	CAD	APPD	DESCRIPTION OF REVISION
0	FR-2202	09/1/11	TFI	TFI	DAF	RELEASED FOR PRODUCTION



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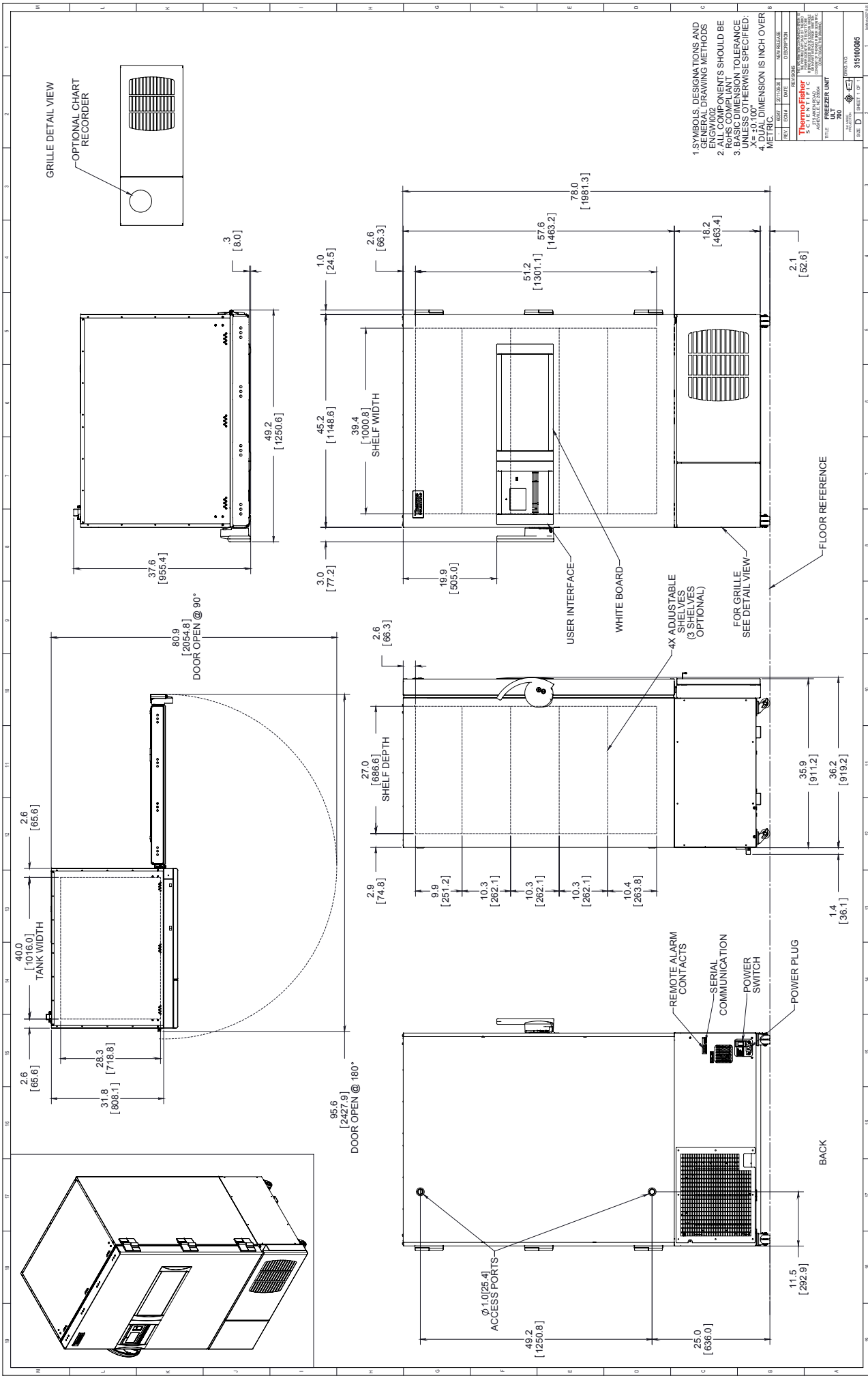
MODEL/PART NAME: 23 CU FT., -86°C UPRIGHT FREEZERS  
 GENERAL DIMENSION DRAWING ULT  
 DATE: 06/1/2011  
 SCALE: N/A

**ThermoFisher**  
**SCIENTIFIC**  
 BOX 648, MARIETTA, OHIO 45759

NOTE: DUAL DIMENSION IS INCH OVER METRIC.

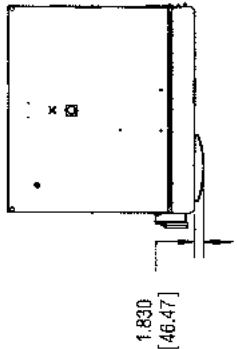
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SIZE	B

SAVES: 2011-11-21

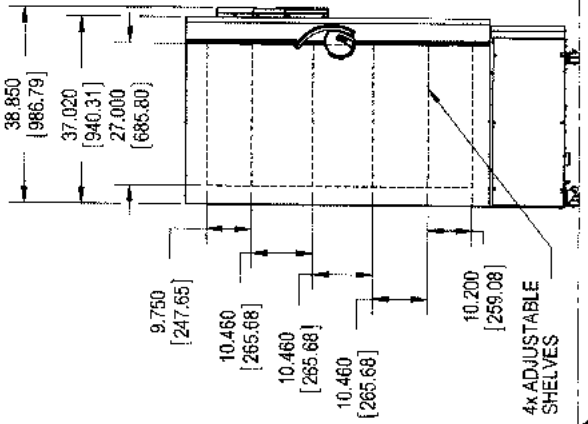
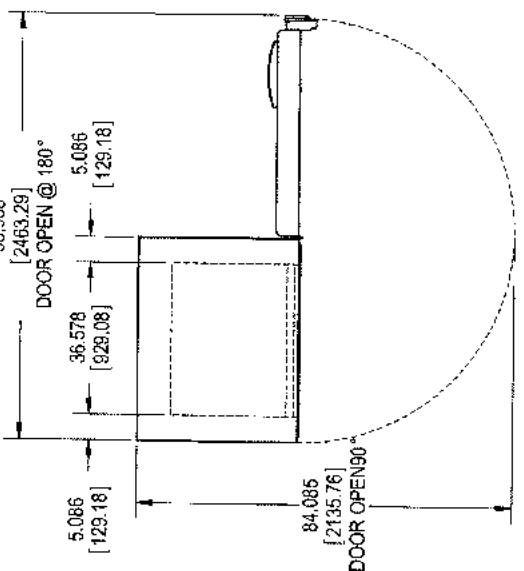


DRAWING NUMBER: 8930-00-1

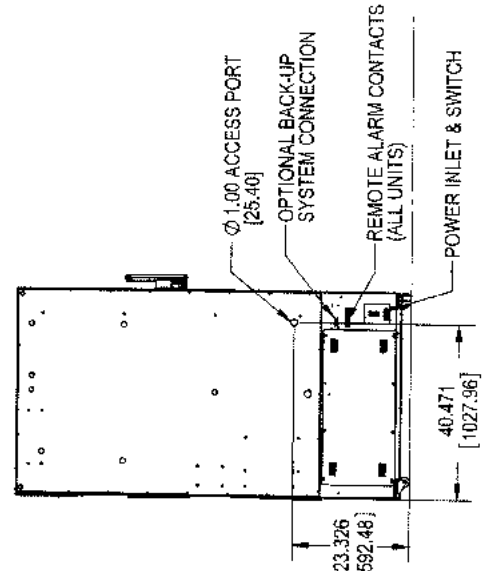
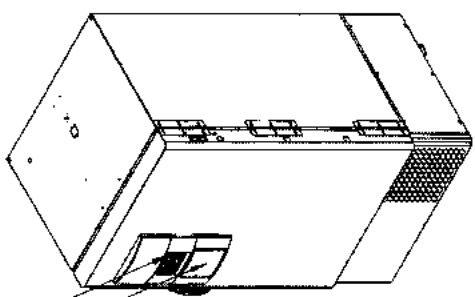
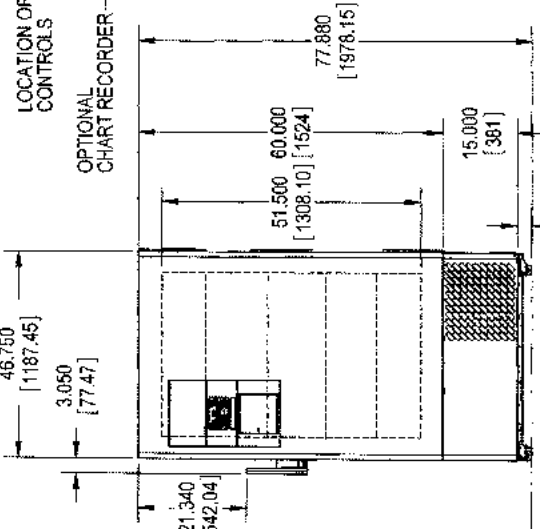
REV	ECN NO.	DATE	BY	CAD	APPD	DESCRIPTION OF REVISION
0	FR-2202	06/11/11	TFI	TFI	JH	RELEASED FOR PRODUCTION



1.830  
[46.47]



LOCATION OF  
CONTROLS  
OPTIONAL  
CHART RECORDER



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MODEL/PART NAME: 28 CU FT. -85°C UPRIGHT FREEZERS

GENERAL DIMENSION DRAWING ULT

DWN: TFI CAD: TFI APPD: SSK DATE: 06/12/11 SCALE: N/A

MATERIAL: N/A

PAINT COLOR: N/A

TOLERANCE UNLESS OTHERWISE SPECIFIED: .005+0.00

UNITS: ANGLES: DECIMAL: .0005+0.00

**ThermoFisher**  
**SCIENTIFIC**

BOX 688, MARIETTA, OHIO 45750

NOTE: DUAL DIMENSION IS INCH OVER METRIC.

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